

1 **Short- and long-term observations of fracture permeability in granite by flow-through**  
2 **tests and a comparative observation by X-ray CT**

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17 **Abstract**

18 To grasp the fracture contact-area variation is a kernel on understanding the permeability  
19 evolution of the fractured rocks. However, there is a lack of studies focusing on measuring the  
20 long-term fracture contact-area variation under different conditions. In this study, a series of  
21 short- and long-term permeability tests under coupled conditions are performed to check the  
22 permeability performance. Permeability is sensitive to confining pressures and shows  
23 temperature dependence in the short-term tests, and it has the irreversible change and time-  
24 dependence in the long-term tests. In order to verify the permeability evolution, by assembling  
25 a triaxial cell with heating capability, a microfocus X-ray CT is developed to observe the  
26 internal fracture structure change under the same conditions with the long-term permeability  
27 tests. The fracture aperture and the fracture contact-area ratio are calculated by CT image  
28 analysis technique. The estimated aperture decreases with increase of the confining pressure,  
29 and it decreases with time under the constant confining pressure. Moreover, the hydraulic  
30 aperture calculated from the CT observation is qualitatively consistent with that measured in  
31 the long-term permeability tests. It is clarified that the fracture contact-area increase with time  
32 under the constant confining pressure corresponds to the permeability decrease in the long-term.

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35 **Keywords:** Permeability, short- and long-term, X-ray CT observation, contact-area ratio and  
36 aperture distribution

37

## 38 **Introduction**

39       The flow behavior in the deep subsurface has been given great attention in recent years.  
40       The coupled thermal-hydraulic-mechanical-chemical (THMC) processes exert a significant  
41       effect on the subsurface fluid flow in geological systems (Ghassemi et al., 2007; Taron et al.,  
42       2010; Frdric et al., 2017). Especially during the geological disposal of high-level radioactive  
43       waste, the stability of the disposal system may be affected by the long-term water flow around  
44       the rock fractures (Rutqvist et al., 2002; Yasuhara et al., 2015; Tsang et al., 2012;). Moreover,  
45       the coupled THMC processes might change the underground water flow paths, and then the  
46       transport of radionuclides will be promoted. Under the coupled interaction, the mechanical  
47       creep (stress corrosion) (Yasuhara et al., 2006; Xu et al., 2016) and the geochemical response  
48       (variation in mineral composition) (Polak and Beeler, 2004; Yasuhara et al., 2006, 2011;  
49       Elkhoury et al., 2013) altering the fracture surface roughness and transforming the long-term  
50       permeability evolution are discussed.

51       Several numerical works focusing on the prediction of the variation in permeability under  
52       the coupled THMC processes have been proposed (Taron et al., 2010; Zhang et al., 2013; Wang,  
53       2017). However, not many laboratory works have addressed the long-term fluid flow behavior  
54       within the rock fractures or the changing of the fracture contact-area under the THMC coupled  
55       processes (Li et al., 2008; Yasuhara et al., 2015; Xu et al., 2016). Polak et al. (2003), Yasuhara  
56       et al. (2015), and Farough et al. (2016) conducted several long-term experiments, and the results  
57       showed that the fracture surface roughness was altered, and the aperture decreased at higher  
58       temperatures, which resulted in the permeability reduction. From the experimental studies, it is  
59       seen that under the coupled processes, the fracture contact-area should be evolved with time.

60 Subsequently, the fluid flowing through the fracture would be disturbed by the evolved fracture  
61 contact-area (Zimmerman et al., 1996; Li et al., 2008; Kishida et al., 2013). However, the  
62 evolution of the fracture contact-area has not been well investigated in the previous studies,  
63 because it is not simple to grasp the fracture contact-area immediately through the laboratory  
64 tests.

65 From the previous works, it is noted that the temporal fracture structure change needs to  
66 be further investigated. X-ray computed tomography (CT) (Van et al., 2001; Yao et al., 2009;  
67 Ketcham et al., 2010) is an effective technique which ~~canis capable of~~ detecting the internal  
68 structure non-destructively and three-dimensionally (Robert et al., 1993; Fan et al., 2018).  
69 Therefore, it is possible to grasp various rock samples with fractures or cracks under the flow  
70 test. (Stephanie et al., 2001; Karpyn et al., 2007, 2009; Dustin et al., 2017; Lu and Kumaria,  
71 2018). The measured data is used to reconstruct a 3D view for illustrating the fracture  
72 heterogeneous distribution (Keller, 1998; Kawakata et al., 1999; Mazumder et al., 2006; Yao et  
73 al., 2009; Richard et al., 2010; Hamed et al., 2016). Moreover, some researchers try to use  
74 microfocus X-ray CT to observe the fracture changing under varying conditions (Polak et al.,  
75 2003; Yasuhara et al., 2015; Okamoto et al., 2017). Polak et al., (2003) studied the fracture  
76 change under different temperatures (20, 80 and 120 °C), the mass removed from the fracture  
77 contact-area and the decrease in hydraulic aperture were investigated. Okamoto et al., (2017)  
78 showed a comparison of the CT scanning images between a sample at room temperature and at  
79 the temperature of 350°C. The quartz distribution illustrated that the flow rate was changed due  
80 to the mineral dissolution or the precipitation generated at 350°C. Yasuhara et al., (2015)  
81 conducted a series of permeability tests with several types of rock samples. Pore structures were  
82 observed through X-ray CT with a comparison between the pre- and post-experiment. The

83 changing pore structures might support the variation in permeability. (Caulk et al., 2016;  
84 Kamali-Asl et al., 2018) found that the aperture would close at higher levels of confining stress.  
85 The higher levels of confining stress resulted in a change in the sensitivity of the variable  
86 fracture contact-area ratio.

87 However, previous laboratory studies have not directly observed the temporal change of  
88 the fracture contact-area under the coupled conditions. There is a lack of interpretation of the  
89 relationship between the long-term permeability changing and the fracture contact-area  
90 variation through laboratory works. Therefore, to further understand the long-term permeability  
91 evolution under coupled processes, this study focuses on several permeability tests under  
92 different conditions, especially for the variation in permeability over the long-term under a  
93 constant confining pressure. An X-ray CT observation as well as the long-term permeability  
94 tests are conducted to obtain the changes of the aperture distribution and the fracture contact-  
95 area. Finally, several indexes, such as the fracture contact-area ratio, the aperture distribution  
96 and a comparison of the hydraulic aperture change, are discussed.

97

## 98 **2. Sample materials and experimental procedures**

### 99 **2.1 Fracture generation and morphology**

100 **Figure 1** presents the geometry of the samples used in this study. Two cylindrical granite  
101 samples, Sample #1 and Sample #2, with a size of  $49.3 \times 101.2$  mm and  $15.2 \times 33.3$  mm,  
102 respectively, are employed. Samples #1 and #2 are used for the permeability tests and the X-  
103 ray CT observation, respectively. Before the experiments, both samples are split by the  
104 Brazilian tensile testing method to create a single fracture along the cylinder axis (Fairhurst,  
105 1964). The mechanical properties of the granite samples are listed in **Table 1**.

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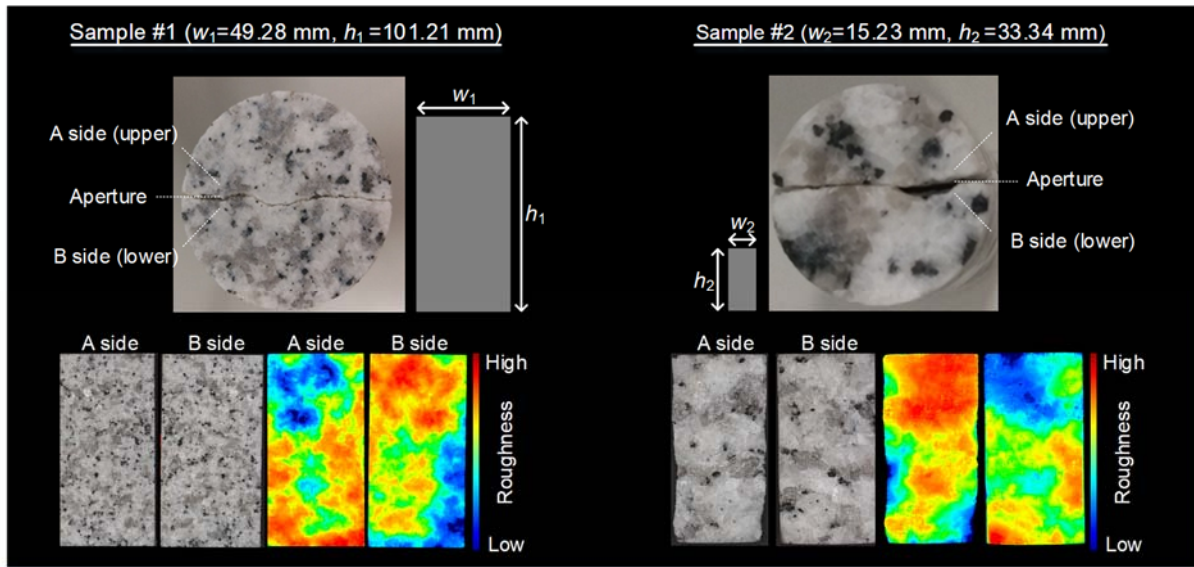
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**Table 1** Mechanical properties of granite sample

| Sample  | Uniaxial<br>compression strength<br>(MPa) | Young's modulus<br>(GPa) | Poisson's ratio | Tensile strength<br>(MPa) |
|---------|---|--------------------------|-----------------|---------------------------|
| Granite | 140.3                                     | 55.4                     | 0.2             | 6.2                       |

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**Fig. 1** Sample experiments and fracture surfaces measured by laser profilometer

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In this study, the initial granite surface roughness before the experiments is evaluated by the JRC values. An optical profilometer is employed, using the pattern projection method (VR-3200, Keyence Corporation in Japan), to grasp the morphology of the fracture surfaces. The profilometer is capable of measuring the surface roughness in a grid pattern through non-contact. The grid size of the elevation measurement is 25  $\mu\text{m}$  and the measurement resolution of the profilometer is 0.1  $\mu\text{m}$ . The color contour maps in **Fig. 1** illustrate the elevations of the surface roughness.

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To clarify the variation in fracture surface roughness, it is necessary to grasp the surface roughness precisely before starting the experiments. Barton's standard of JRC (Joint Roughness

123 Coefficient) profiles is selected as the comparison index. Typically, the JRC value ranges from  
 124 0 to 20 in the shear behavior of the rock joints (Barton et al., 1977,1997). The JRC values can  
 125 be calculated with the dimensionless parameter  $Z_2$  (Tse et al., 1979), which has been widely  
 126 discussed and is defined as follows (Zhang and Dimadis, 2014; Yin et al., 2017; Yong et al.,  
 127 2018):

$$Z_2 = \left[ \frac{1}{M-1} \sum_{i=1}^{M-1} \left( \frac{\Delta y}{\Delta x} \right)_i^2 \right]^{\frac{1}{2}} \quad (1)$$

129 where  $\Delta x$  is the sampling interval,  $\Delta y$  is the difference between two adjacent points and  $M$  is  
 130 the number of sampling points along the length of the fracture surface. Subsequently, the JRC  
 131 values can be evaluated by

$$JRC = 60.32Z_2 - 4.51(\Delta x = 0.25 \text{ mm}) \quad (2)$$

133 **Table 2** lists the  $Z_2$  and JRC values for both surfaces of Samples #1 and #2. The JRC  
 134 values for the two samples are around 20. The initial values are used to evaluate the difference  
 135 in surface roughness between the pre- and post-experiment.

136  
 137 **Table 2** Comparison of  $Z_2$  and JRC values with profilometer equipment (VR-3200)

|           | (JRC=60.32* $Z_2$ -4.51) 0.25 mm |        |
|-----------|----------------------------------|--------|
|           | A side                           | B side |
| Sample #1 | 19.9                             | 20.2   |
| Sample #2 | 22.8                             | 20.4   |

138

## 139 2.2 Transient pulse method

140 To investigate the permeability evolution under various conditions, the transient pulse  
 141 method is applied to measure the permeability value (Hsieh et al., 1981). The pore water  
 142 pressures between the upstream and downstream in Eq. (3) are different. The tank is located  
 143 upstream of the sample. The pressure is increased by the operator in tiny increments of the

144 injected water, then the permeability is measured from the pressure gradient generated between  
 145 the two ends of the sample and the lapse of time. With the passage of time, the difference in  
 146 pressure will reach a new equilibrium, given as:

$$\begin{aligned}
 147 \quad P_d &= \frac{V_u P_u^0 + V_d P_d^0}{V_u + V_d} - \frac{V_u}{V_u + V_d} (P_u^0 - P_d^0) \exp\left(-\frac{k}{\beta \mu} \frac{A}{L} \frac{V_u + V_d}{V_u V_d} t\right) \\
 148 \quad P_u &= \frac{V_u P_u^0 + V_d P_d^0}{V_u + V_d} - \frac{V_d}{V_u + V_d} (P_u^0 - P_d^0) \exp\left(-\frac{k}{\beta \mu} \frac{A}{L} \frac{V_u + V_d}{V_u V_d} t\right) \quad (3)
 \end{aligned}$$

149 Where  $P_u$  and  $P_d$  are the upstream and downstream pressure, respectively, and  $V_u$  and  $V_d$   
 150 are the volume of two reservoirs, i.e., the volume of each reservoir is equal to 0.001 m<sup>3</sup> in this  
 151 work.  $A$  and  $L$  are the cross-section and the length of the sample, respectively,  $\mu$  is the viscosity  
 152 of the fluid and  $\beta$  is the fluid compressibility.  $\mu$  and  $\beta$  are temperature-dependent (Walsh,  
 153 1981). Then the coefficient of permeability can be evaluated from the following equations:

$$154 \quad \ln((P_u - P_d)/2) = \ln\left(\frac{V_d}{V_u + V_d} (P_u^0 - P_d^0)\right) - \frac{k}{\beta \mu} \frac{A}{L} \frac{V_u + V_d}{V_u V_d} t \quad (4)$$

$$155 \quad \alpha = -\frac{k}{\beta \mu} \frac{A}{L} \frac{V_u + V_d}{V_u V_d} \quad (5)$$

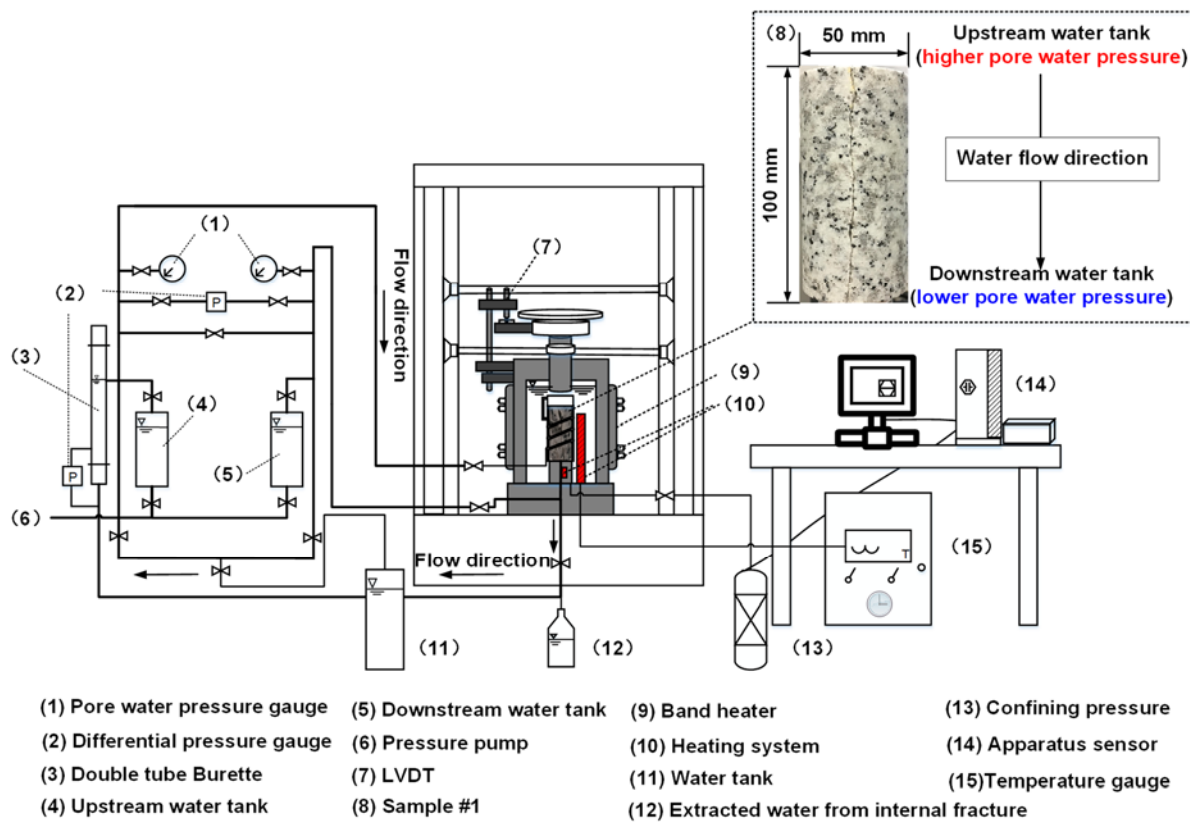
156 In Eq. (5),  $A$  represents the cross-sectional area of the sample. In this study, however, the fluid  
 157 only flows through the fracture aperture. Therefore, cross-section  $A$  should be changed to  $A =$   
 158  $b \times W$ , where  $W$  is the width of the sample and  $b$  is the hydraulic aperture. Consequently, based  
 159 on the cubic law, Eq. (5) is rewritten as follows:

$$160 \quad \alpha = -\frac{b^3}{12\beta \mu} \frac{W}{L} \frac{V_u + V_d}{V_u V_d} \quad (6)$$

161 **Figure 2** shows a schematic illustration of the test apparatus. The granite sample (Sample  
 162 #1) is installed in a triaxial cell. The maximum pressure and temperature that can be prescribed  
 163 are 20.0 MPa and 200°C, respectively. The sample is sealed with a heat-shrinkable tube and  
 164 fixed to pedestals inside the vessel. A thermocouple is installed to measure the temperature  
 165 close to the sample. Two water storage tanks supply water that passes through Sample #1.



166 Differential pore water pressure gauges are used to measure the difference in pressure between  
 167 the upstream and the downstream. To check the internal fracture effluent after each permeability  
 168 test, distilled water is used as the injection fluid in this experiment.  
 169



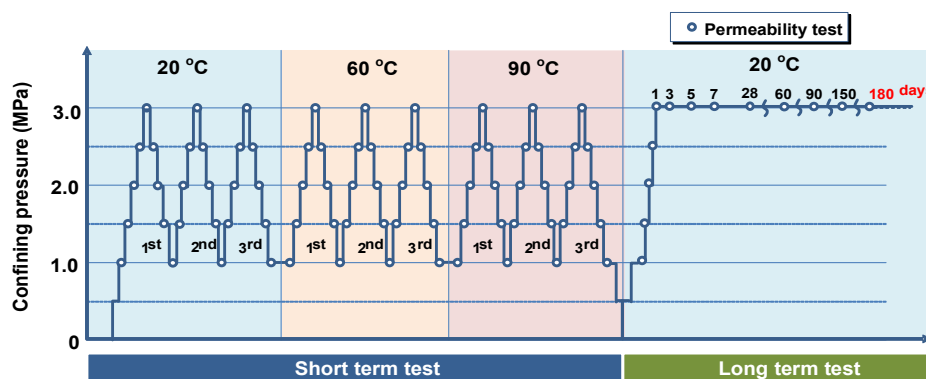
170  
 171 **Fig. 2** Triaxial apparatus and Sample #1 installed in internal cell. The upstream and downstream pore water  
 172 pressures are prescribed to the top and bottom of Sample #1, respectively

173  
 174 **2.3 Experimental procedure of the permeability tests (Sample #1)**

175 **Figure 3** shows the experimental procedure of permeability tests. A series of permeability  
 176 tests is conducted over both short- and long terms to observe the temporal changes in  
 177 permeability. Short-term permeability tests are performed for 6 days, while long-term tests are  
 178 performed for 180 days after the short-term tests finished. The short-term tests are carried out  
 179 at three different temperatures of 20, 60, and 90 °C. Three cycles of the loading-to-unloading  
 180 process under each confining pressure are conducted. The confining pressure is increased in

181 increments of 0.5 MPa from 1.0 to 3.0 MPa. The permeability tests are performed  
182 simultaneously at each confining pressure.

183 After the short-term tests, the temperature is decreased to 20 °C. Then the long-term test  
184 is conducted under the confining pressure of 1.0 MPa and the temperature of 20 °C. The  
185 confining pressure is increased from 1.0 to 3.0 MPa again and then keeps constant at 3.0 MPa  
186 for 180 days. During this process, the permeability is measured at the prescribed time intervals,  
187 as shown in **Fig. 3**. Simultaneously, the effluent is obtained from the internal fracture, to  
188 measure the element concentrations after each permeability test. The differential pore water  
189 pressures, fluid viscosity, and fluid compressibility are listed in **Table 3**. The permeability  
190 values are measured under various confining pressure conditions during the loading-to-  
191 unloading processes. **Fig.4** shows an example of the permeability tests. A difference in pressure  
192 between the upper and lower ends of the granite Sample #1 is produced with a slight change in  
193 the pore water pressures. Then, the permeability can be measured by the temporal change in the  
194 pressure difference during the elapsed time (600 seconds).



195  
196  
197

**Fig. 3** Experimental procedure for short- and long-term permeability tests

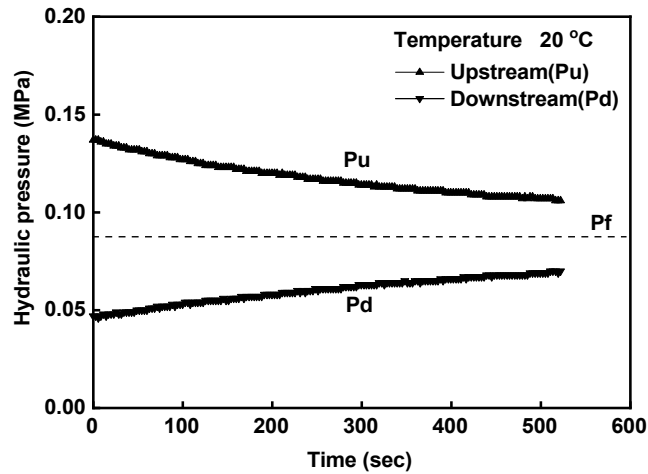


Fig. 4 Temporal changes in upstream and downstream water pressures in transient pulse tests

#### 2.4 Non-destructive observation of fracture with microfocus X-ray CT (Sample #2)

In this study, a small-scale triaxial cell coupled with the X-ray CT equipment is developed to observe the change of the fracture structure in the rock under various confining pressures and temperatures, as shown in **Fig.5**. A lucid acrylic cell, without steel pillars, is utilized to make the cell as transparent to the X-ray as possible in order to avoid the X-ray attenuation by the steel (Otani et al., 2002). With a heater installed in the cell, the maximum confining pressure and temperature can be applied up to 3 MPa and 120 °C, respectively. The X-ray CT equipment is KYOTO-GEO $\mu$ XCT (TOSCANER-32250 $\mu$ hdk) (Kido et al., 2020). The minimum focus size is 4  $\mu$ m and the resolution performance is 5  $\mu$ m.

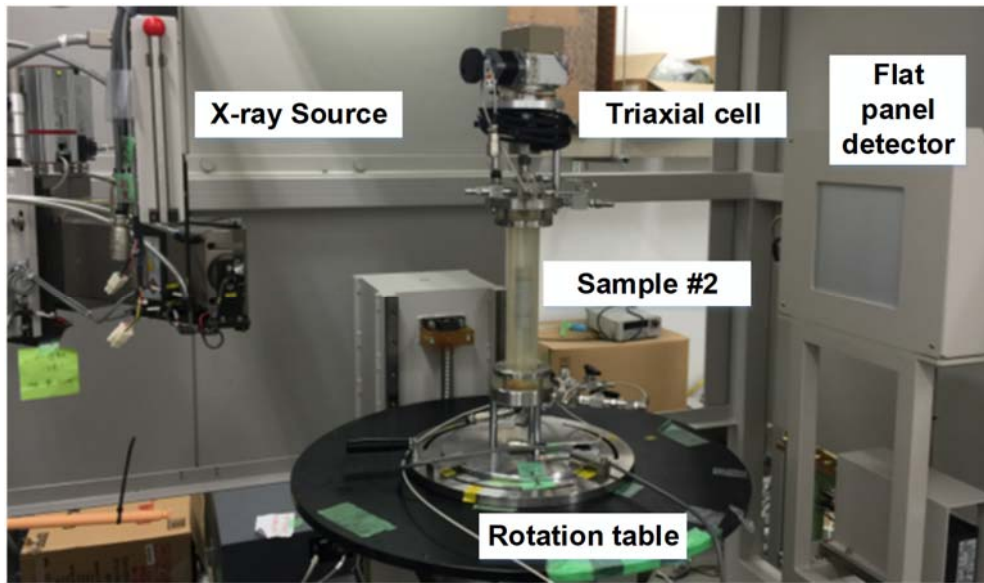
**Figure 6** shows the CT slices scanned by the X-ray CT. A CT slice is composed of a number of voxels which include CT values determined based on the amount of the X-ray attenuation, depending on the material density. In the CT slice, white and black portions correspond to higher and lower density materials, respectively. The internal fracture structure of Sample #2 can be visualized in 3D by cumulating the CT slices obtained at each height of the sample.

216

**Table 3** Experimental conditions of permeability tests

|                                   |                  |                 |                  |       |       |
|-----------------------------------|------------------|-----------------|------------------|-------|-------|
| Confining pressure (MPa)          | 1.0              | 1.5             | 2.0              | 2.5   | 3.0   |
| Pressure at upstream tank (MPa)   | 0.200            | 0.250           | 0.300            | 0.350 | 0.400 |
| Pressure at downstream tank (MPa) | 0.150            | 0.175           | 0.200            | 0.225 | 0.250 |
| Fluid viscosity ( $10^{-10}$ Pas) | 1.0017 (T=20 °C) | 0.467 (T=60 °C) | 0.0328 (T=90 °C) |       |       |
| Compressibility ( $10^{-3}$ Pa)   | 4.5883 (T=20 °C) | 4.3 (T=60 °C)   | 4.6 (T=90 °C)    |       |       |

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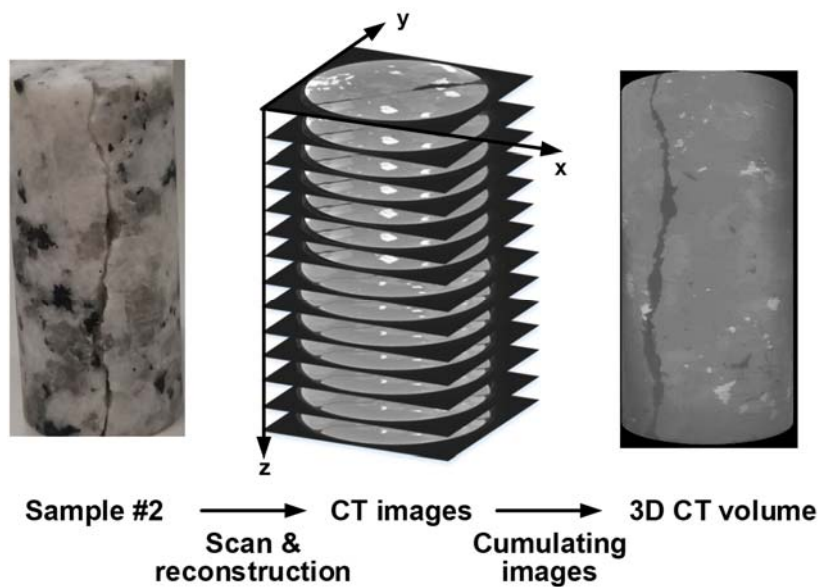


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219

**Fig. 5** Triaxial cell coupled with the microfocus X-ray CT equipment

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223

**Fig. 6** 3D CT volume of Sample #2

224 The scanning procedure for Sample #2 is as follows. Firstly, the initial fracture aperture is  
225 scanned at the as-received (dry) condition of 0 MPa and 20 °C. Secondly, the sample is fixed  
226 in the triaxial cell under the water-saturated condition. The confining pressure is increased up  
227 to 3.0 MPa and the fracture aperture is scanned. These two scans are performed in one day.  
228 Subsequently, the confining pressure is kept constant at 3.0 MPa, and CT scans are performed  
229 at 14th, 30th, 60th, 90th, 120th, 150th and 180th days, corresponding to the same procedure  
230 with the permeability test of Sample #1. One CT slice comprises 1024 voxels in each  $x$ - and  $y$ -  
231 direction as shown in **Fig. 6**. At 0 MPa, the voxel size is 15.1  $\mu\text{m}$ , 15.1  $\mu\text{m}$  and 17.0  $\mu\text{m}$  in  $x$ -,  
232  $y$ - and  $z$ -directions. At 3.0 MPa, the voxel size is 18.4  $\mu\text{m}$ , 18.4  $\mu\text{m}$  and 21.0  $\mu\text{m}$ , respectively.

233

## 234 **2.5 Methodologies of CT image analysis**

### 235 **2.5.1 Segmentation**

236 In order to detect the fracture aperture position in the CT volume scanning Sample #2, the  
237 segmentation of the CT images is required. As mentioned above, a CT slice is an assembly of  
238 discrete voxels including representative CT values for each material and thus, the segmentation  
239 can be performed based on the CT values. In the present study, the rock phase and the void  
240 phase in the CT images are distinguished from each other using a region growing method  
241 (Rosenfeld et al., 1982), which is implemented with the 3D image analysis software  
242 VGstudioMax3.1 (Volume Graphics GmbH) (Higo et al., 2014). This method examines the CT  
243 values of the neighboring voxels of initial seed voxels and determines whether the voxel  
244 neighbors should be added into the region of the seed groups. The regions are extended from  
245 the seed voxels to adjacent voxels depending on a region membership criterion, i.e., tolerance.  
246 The essential factors in the region growing method are initial seed voxel selection (3D location

247 and CT value), a tolerance selection (range in CT values), and voxel connectivity for  
248 examination.

249 **Figure 7** shows the outline of the image processing in this study. The procedure of the  
250 segmentation using the region growing method is as follows:

- 251 1) Assuming that a histogram of CT values for a homogeneous material follows a normal  
252 distribution, a mean CT value of the rock phase,  $\mu_p$ , is obtained by analyzing the CT values  
253 using the VGstudioMax3.1. In this study, the mean rock CT value of 151 is chosen as the  
254 initial seed of the rock phase.
- 255 2) Similarly, the mean CT value of the void phase,  $\mu_v$ , of 48 is obtained. A threshold between  
256 the rock phase and the void phase is an intersection of the CT value histogram (see in **Fig.**  
257 **7(a)**). In this study, the threshold value is 82.
- 258 3) The tolerance of the rock phase,  $T$ , is the difference between the mean CT value of the rock  
259 phase and the threshold determined in the above step, i.e., the tolerance is 69. Adjacent  
260 voxels to the initial seed voxel whose CT values are from  $\mu_p - T$  to  $\mu_p + T$  (i.e., from 82 to  
261 220) are assimilated into the rock phase seed groups. The voxel connectivity of the region  
262 growing is a 26-connected neighborhood in 3-dimensional, which ensures the continuity  
263 of adjacent voxels to the initial seed voxel in  $x$ -,  $y$ - and  $z$ -directions (see **Fig. 7(b)**).
- 264 4) This process is iterated on until all of the neighboring voxels of the rock phase are  
265 assimilated and no further significant change is found. The remaining voxels after the  
266 iterative process of the region growing for the rock phase are regarded as the void phase, a  
267 segmentation CT image for the rock and void phase is depicted in **Fig. 7(c)**.

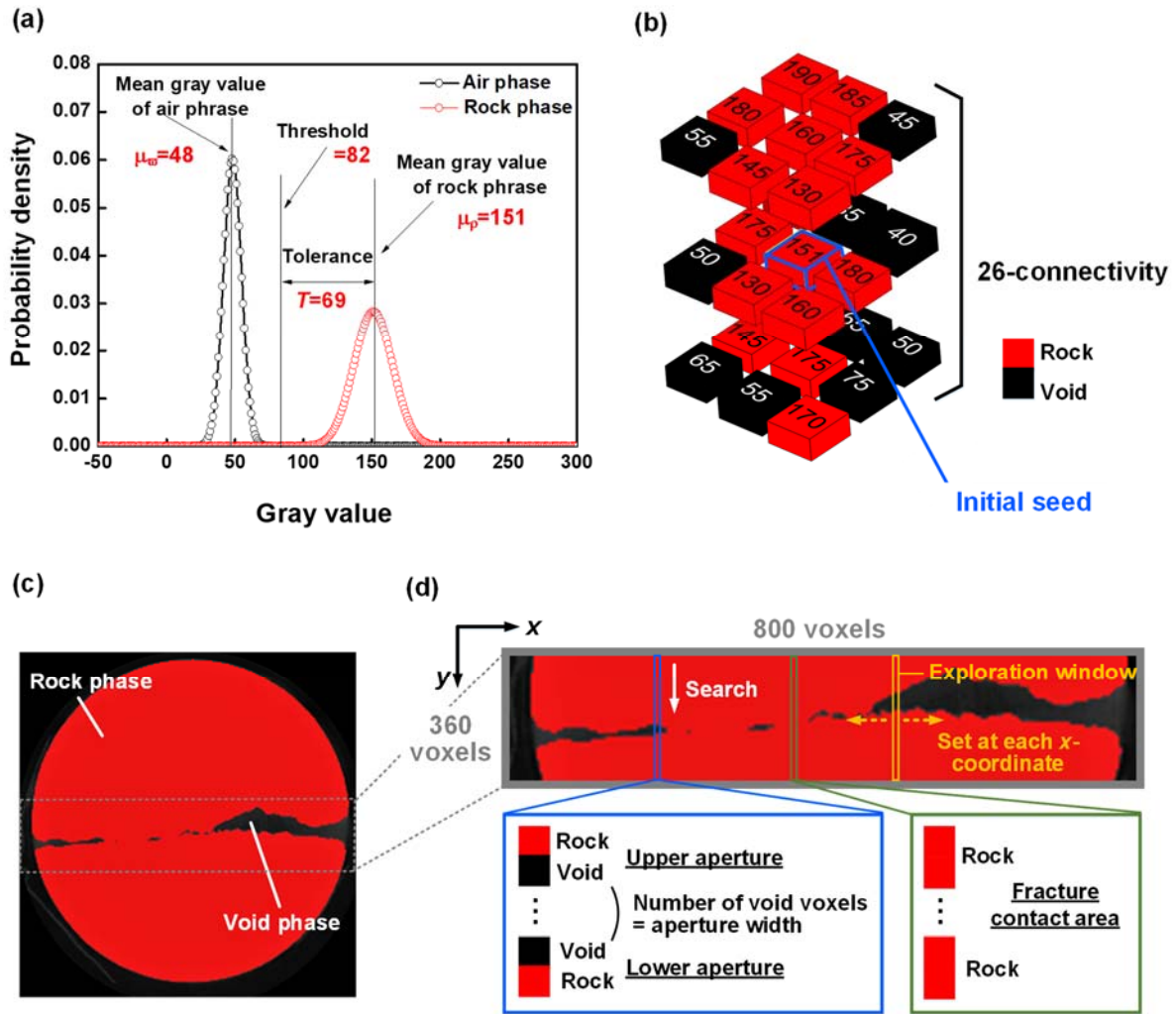
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269

## 270 **2.5.2 Detection of fracture aperture and calculation of fracture contact-area**

271 **Figure 7(d)** shows the detection method of the internal aperture position from a CT  
272 segmentation image. A rectangular area, 800 voxels in  $x$ -direction and 360 voxels in  $y$ -direction  
273 is extracted from the segmentation image as an analysis region where the whole aperture part  
274 exists. At first, an exploration window, 1 voxel in  $x$ -direction and 360 voxels in  $y$ -direction is  
275 defined. Second, each voxel in the exploration window is searched in the  $y$ -direction. Then, the  
276 position where a voxel of the rock phase locates above a voxel of the void phase is detected as  
277 the upper aperture, whereas the position where a voxel of the rock phase locates below a voxel  
278 of the void phase is detected as the lower aperture. As shown in **Fig. 7(d)**, the aperture width  
279 corresponds to the number of voxels for the void phase between the upper and lower apertures.  
280 If there is no voxel for the void phase in the exploration window, this position is regarded as  
281 the fracture contact-area. The voxel search is performed for the exploration window set at each  
282  $x$ -coordinate in the analysis region. In this study, the number of contact-area positions to the  
283 number of voxels in the  $x$ -direction of the analysis region is defined as the fracture contact-area  
284 ratio.

285 The same processing as mentioned above is applied to all of the segmentation images,  
286 providing the internal fracture information, i.e., aperture distribution and the fracture contact-  
287 area ratio of Sample #2.



288  
 289 **Fig. 7** Segmentation process for rock phase and void phase by region-growing technique:(a) determination of region  
 290 growing, (b) schematic illustration of region growing in 3D, voxel connectivity of 26-connected neighborhood (c)  
 291 segmentation image in 2 dimensional and (d) voxel that includes rock and void phases (aperture detection method)  
 292 and fracture contact-area in aperture position

293

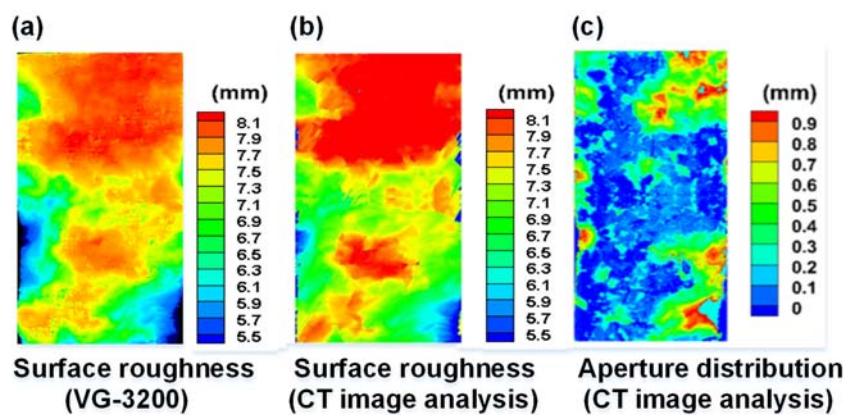
### 294 2.5.3 Validation of the CT image analysis

295 **Figures 8(a)** and **(b)** present the contour map of the surface roughness (Sample #2)  
 296 measured by the optical profilometer (VG-3200) and that are analyzed by the CT image analysis  
 297 method. For both of them the data are obtained at 0 MPa, as-received condition before starting  
 298 the experiment. It is clarified that the extracted surface roughness of Sample #2 under the two  
 299 methods is qualitatively similar to each other. **Figure 8(c)** shows the contour map of the aperture  
 300 distribution measured by the CT image analysis. This distribution is derived from the aperture



301 width for each CT slice, i.e., the difference between the upper aperture and the lower aperture,  
 302 as mentioned in Subsection 2.5.2. The blue part of the contour map in **Fig. 8(c)** represents the  
 303 fracture contact-area. The initial arithmetical mean aperture (hereinafter called the mechanical  
 304 aperture) and the fracture contact-area ratio without the confining pressure are 0.253 mm and  
 305 5.4%, respectively.

306



307 **Fig.8** Contour maps of surface roughness (A-Side as mentioned in **Fig.1**) and aperture distribution of Sample #2 at 0 MPa. (a)  
 308 surface roughness obtained through VG-3200 pattern projection, (b) surface roughness measured through the CT image  
 309 analysis, and (c) aperture distribution measured through the CT image analysis.

310

### 311 3. Experimental results

#### 312 3.1 Short-term permeability tests

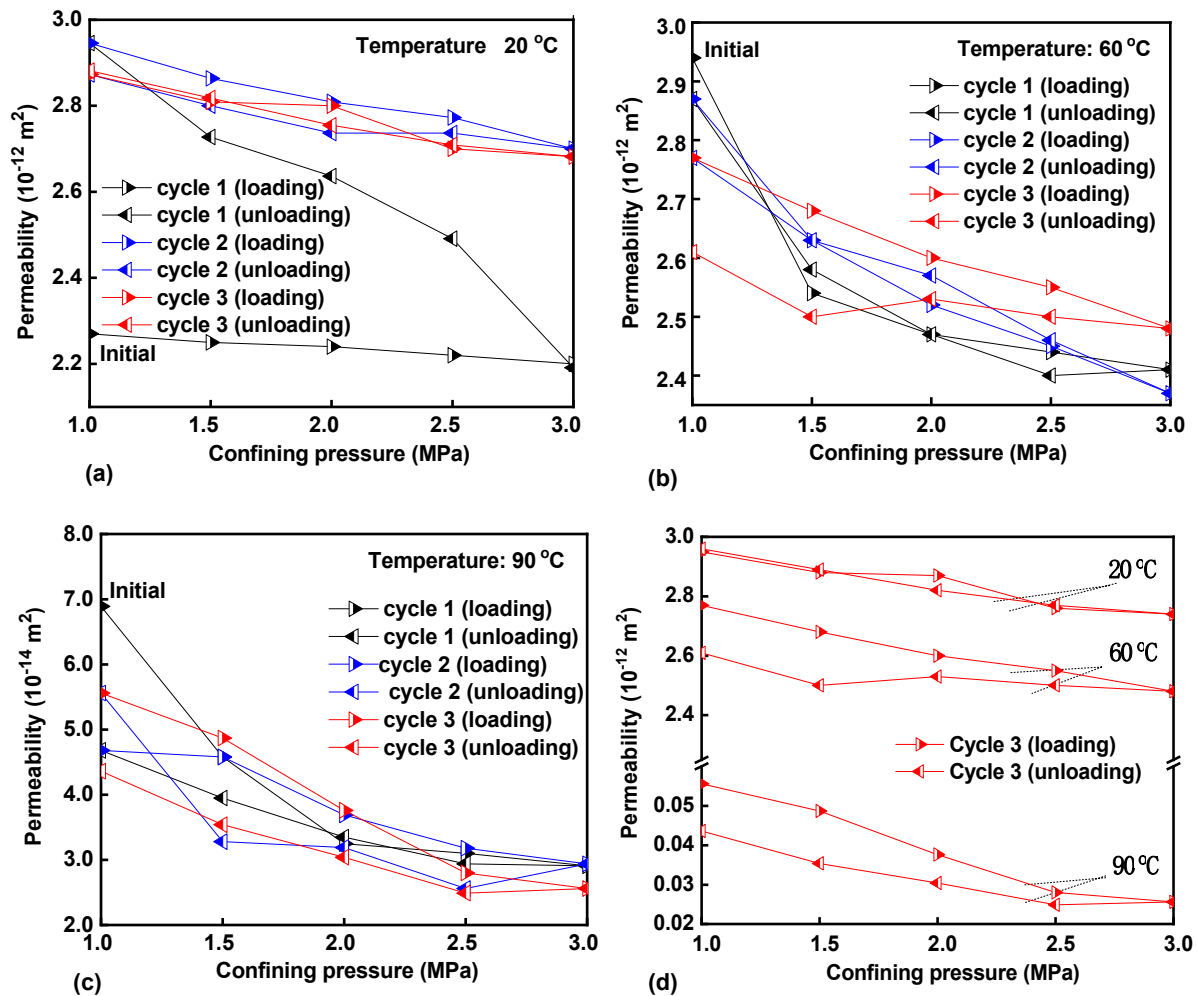
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314 **Figure 9** illustrates the changes in permeability with different levels of confining pressure  
 315 and temperature. In the figure, the right-pointing and left-pointing triangles represent the  
 316 loading and unloading processes, respectively. At 20 °C (**Fig. 9(a)**), the permeability decreases  
 317 with the increase in confining pressure, and increases with the decrease in confining pressure  
 318 throughout the tests. In particular, the permeability changes greatly and irreversibly in the first  
 319 cycle of the loading-to-unloading process. Before the first cycle, the fractures may be ill-mated,  
 320 and the matedness is improved through the first cycle. In the second and third cycles, reversible

321

322 hysteresis curves are observed.

323         At 60 and 90 °C (**Figs. 9(b)** and **(c)**), the similar changes in permeability that alter with the  
324 confining pressure, are observed. Namely, the permeability shows the reversible behavior in the  
325 loading-unloading processes. The results indicate that mechanical compaction should exert a  
326 significant influence on the permeability. The impact may be greater than the influence of the  
327 prescribed temperature. However, the measured permeability also shows a clear temperature-  
328 dependence; it decreases with the increase in temperature as shown in **Fig. 9(d)**. In this figure,  
329 the third cycle of the permeability variation at each temperature is selected for the comparison.  
330 At 90 °C, the permeability decreases from the order of  $10^{-12}$  to that of  $10^{-14}$  m<sup>2</sup>, which is much  
331 smaller than that at 20 and 60 °C. It is inferred that thermal expansion results in the aperture  
332 decrease and the fracture contact-area increase. Moreover, other potential factors, such as the  
333 pressure solution or geochemical responses, might function against the mechanical deformation  
334 and induce irreversible permeability behavior (Polak et al., 2003,2004; Yasuhara et al.,  
335 2006,2015).



336

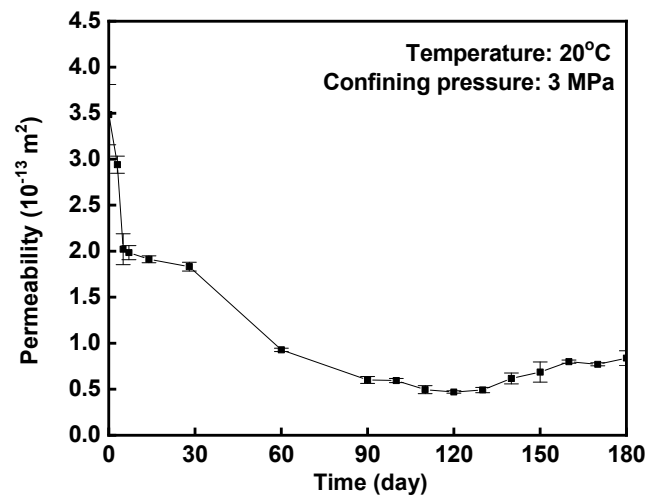
337 **Fig. 9** Short-term test results: (a) permeability variation at 20 °C, (b) permeability variation at 60 °C, (c) permeability  
 338 variation at 90 °C and (d) permeability variation in third cycle under different temperature conditions  
 339

### 340 3.2 Long-term permeability tests

341 Before the commencement of the long-term tests, the permeability increases from 4.36  
 342  $\times 10^{-14} \text{ m}^2$  to  $4.18 \times 10^{-13} \text{ m}^2$  after the temperature is decreased from 90 to 20 at 1.0 MPa. This  
 343 change is attributed to the increase in pore volume due to the decrease in temperature.

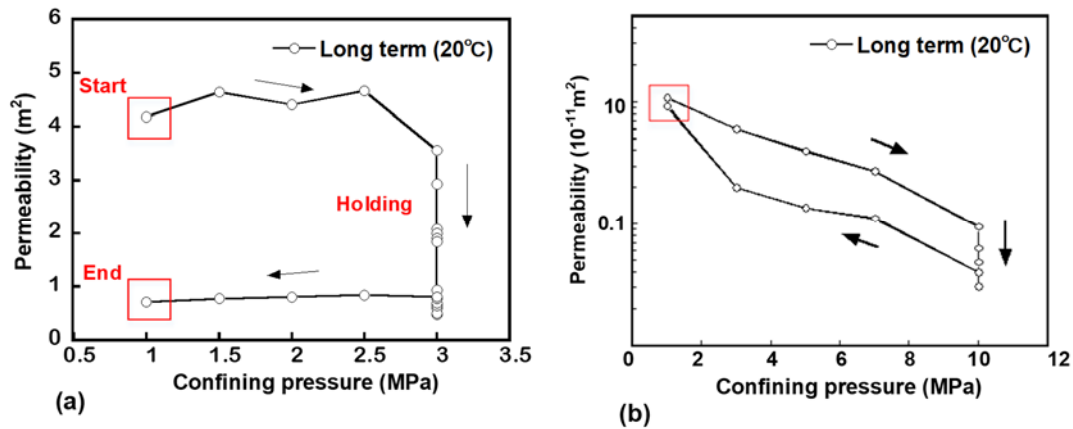
344 **Figure 10** shows the temporal change in the permeability measured in the long-term tests  
 345 under the constant confining pressure of 3.0 MPa. The initial permeability is  $3.7 \times 10^{-13} \text{ m}^2$ . The  
 346 permeability drops sharply within several days and decreases about 45 % of the initial value.  
 347 This change can be interpreted as the mechanical compaction changing the permeability.

348 Subsequently, permeability resumes its decrease after a short stable period. The permeability  
349 decreases to 87 % of the initial value between 30 and 120 days. Creep deformation might lead  
350 to a reduction in permeability when the fluid passes through a stressed fracture. From 120 to  
351 180 days, the permeability shows a slight increase. At the end of the 180-day test, the  
352 permeability value reaches to  $7 \times 10^{-14} \text{ m}^2$ .



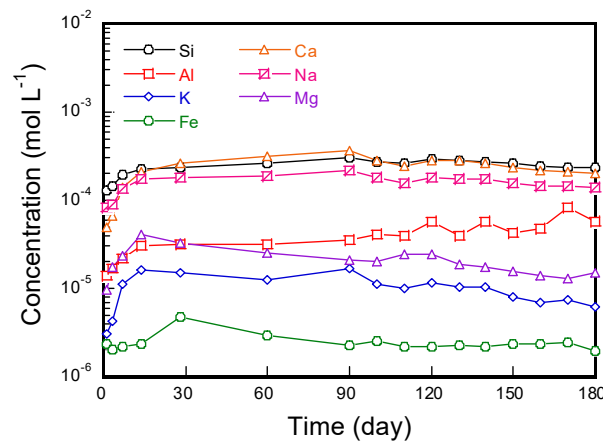
353  
354 **Fig. 10** Change in permeability with time (Sample #1) at 20 °C, 180 days

355 At the end of the long-term permeability tests, the confining pressure has been unloaded  
356 to 1.0 MPa. The permeability evolution during the long-term cycle of the loading-to-unloading  
357 process after the 180-day holding is shown in **Fig. 11(a)**. Although the permeability within the  
358 unloading process shows a little change, an irreversible reduction in permeability is observed  
359 between the onset and the end of the test. This trend is incongruous with the observation made  
360 by Yasuhara et al. (2013) (**Fig. 11(b)**), in which the long-term permeability test was conducted  
361 under a constant confining pressure of 10.0 MPa, at 20 °C for 35 days.



362  
363 **Fig. 11** Comparison of changes in permeability between (a) current work and (b) Yasuhara et al. (2013)

364 The irreversible permeability change is supposed that the propping asperities of the  
365 fracture surfaces are truncated under a relatively long confining condition probably induced by  
366 mechanical crushing and/or geochemical reactions such as pressure solution (Yasuhara et al.,  
367 2015). To verify the geochemical response within the fracture, the effluent from the internal  
368 fracture is checked about the element composition change with the ICP analysis. The element  
369 composition and the concentration results are shown in **Fig. 12**. The concentrations of Si, Na  
370 and Ca are larger than the others. It can be seen from the results of 0 to 30 days that the  
371 concentrations of each element become larger. From 30 to 180 days, those elements show a  
372 slight fluctuation. These results verify that the pressure solution may generate under a long-  
373 term confining condition, and it may evolve the fracture topography with time.



374  
375 **Fig. 12** Evolution of mineral composition in long-term permeability test at 20 °C, 180 days

### 376 **3.3 Observation results using microfocus X-ray CT**

#### 377 **3.3.1 Variation in aperture distribution**

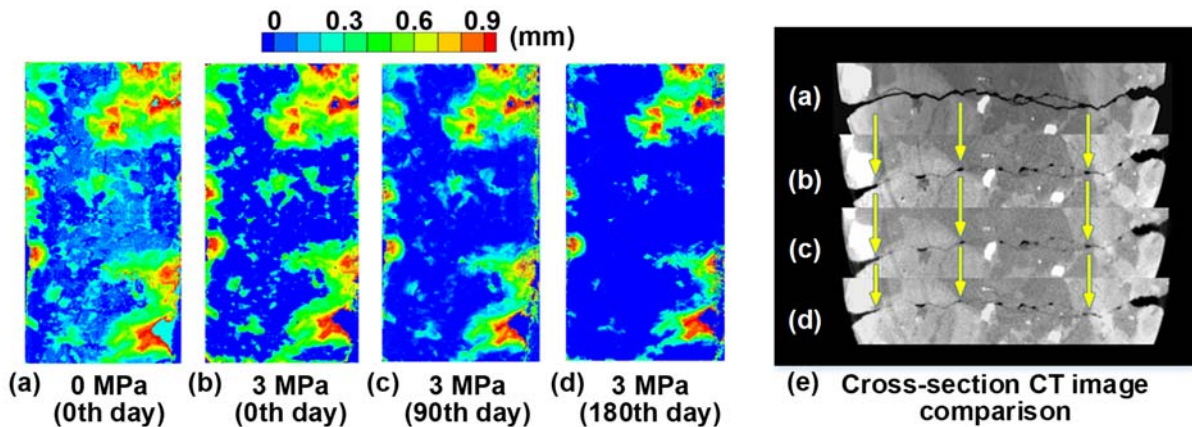
378 The irreversible permeability reduction observed in the long-term tests indicates that the  
379 fracture aperture distribution will be permanently changed with time. As the important  
380 parameters for characterizing the flow paths within the fracture, the geometries of the fracture  
381 surfaces and fracture contact-area are investigated by the X-ray CT technique.

382 **Figure 13** shows the temporal change in the aperture distribution evaluated by the CT  
383 image analysis. In **Figs.13 (a)** and **(b)**, it is clearly seen from the contour maps that the blue-  
384 colored area (i.e., fracture contact-area) increases with the increase of the confining pressure.  
385 When closely observing **Figs. 13 (b)** to **(d)**, the blue-colored area seems monotonically  
386 increasing with time. Namely, the fracture contact-area is enlarged. In **Fig.13 (e)**, the aperture  
387 decrease with time can be clearly observed from the CT images, and it is corresponding to the  
388 image analysis results.

389 Therefore, the fracture contact-area increase can be explained by two processes. In the first  
390 process, the fracture aperture is drastically decreased due to the mechanical compaction by the  
391 increased confining pressure, which is similar to the permeability behavior observed in the  
392 short-term tests. In the second process, the creep deformation induces the long-term alteration  
393 of the fracture roughness surface under the constant confining pressure. Obviously, the second  
394 process also results in the long-term evolution of the fracture permeability.

395

396



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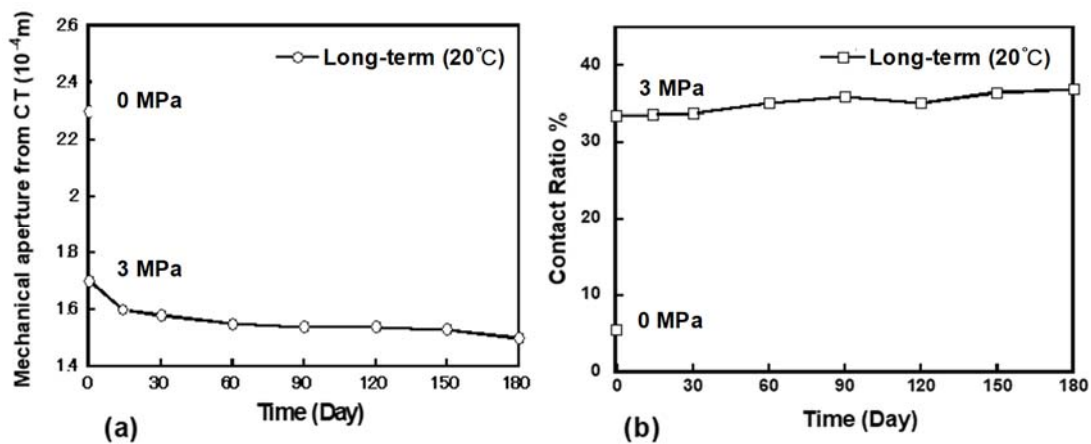
**Fig. 13** Temporal changes in aperture variation evaluated from CT analysis in long-term tests: (a) initial value of mean aperture at 0 MPa, (b) aperture variation after 0 days at 3.0 MPa, (c) after 90 days; (d) after 180 days and (e) cross-section CT images comparison from (a) to (d)

### 403 3.3.2 Quantification of mechanical aperture and fracture contact-area ratio

404 **Figure 14(a)** and **(b)** present temporal changes in the mean values of the mechanical aperture  
405 and the fracture contact-area ratio calculated by the image analysis, respectively. The aperture  
406 value drastically decreases from 0 MPa to 3.0 MPa and the contact-area ratio increases from  
407 5% to 33%. At a constant confining pressure of 3.0 MPa, the mechanical aperture decreases  
408 gradually to 7.5 % from 0 to 90 days. Then, it further decreases to 10 % from 90 to 180 days.  
409 Simultaneously, the tendency for the fracture contact-area ratio increasing is confirmed. The  
410 contact-area ratio reaches approximately 37 % at 180 days.

411 Previous studies used numerical methods to evaluate the fracture contact-area ratio.  
412 Yasuhara et al. (2004) predicted the fracture contact-area ratio less than 30 % at 2.73 MPa and  
413 higher temperatures ranging from 80 to 150 °C. In contrast, the contact-ratio value evaluated in  
414 this study is larger than 30 % at 3.0 MPa - the calculated value tends to be relatively  
415 overestimated compared to those predicted values in the previous studies. The possible reasons  
416 for the discrepancy may be due to the accuracy of the image analysis method as introduced in  
417 section 2.5.2, the spatial resolution of CT images, and so on. It is certain, however, that no

418 previous laboratory study other than the current study has verified the long-term change in the  
 419 actual contact ratio value with/without confining pressure. Moreover, the tendency for the  
 420 contact-area ratio increase calculated by the image analysis is in good agreement with the  
 421 decrease in the fracture aperture observed by the CT scan. In this viewpoint, the experimental  
 422 observation of the fracture contact-area ratio is one of the variable findings in this study, which  
 423 successfully captures an actual phenomenon of the rock fracture evolution, i.e., the fracture  
 424 contact-area increasing under the constant confining pressure.



425  
 426 **Fig. 14** Temporal changes in (a) mean aperture value and (b) contact-area ratio variation from CT image analysis  
 427

## 428 4. Discussions

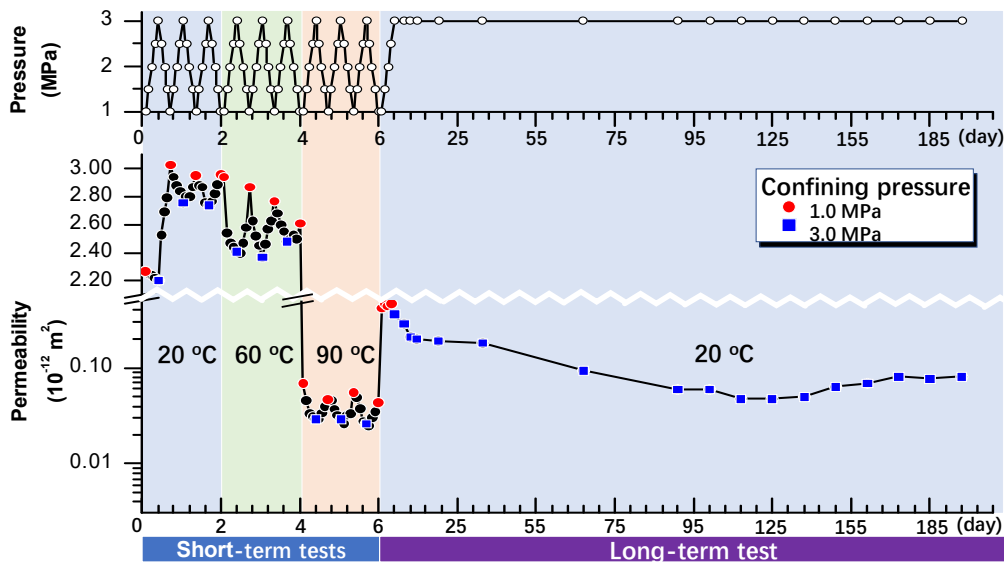
### 429 4.1 Short- and long-term permeability tests

430 **Figure 15** shows a summary of the permeability tests, in which red points represent the  
 431 permeability values at 1.0 MPa, and blue points are the values at 3.0 MPa. It is clearly seen that  
 432 permeability is sensitive to the confining pressures in the short-term. The permeability value at  
 433 1.0 MPa is larger than that at 3.0 MPa. Then the temperature was then reduced to 20 °C again  
 434 after the short-term tests. The permeability value seems increase, however, it cannot reach the  
 435 initial value of the short-term test at 20 °C. Moreover, the permeability is not sensitive to the



436 confining pressure when reloading the confining pressure again.

437 At the long-term tests, the permeability decreases remarkably in the early period, which is  
438 probably resulted from the mechanical compaction due to an increment of confining pressure  
439 (see CT images **Figs. 13(a)** and **(b)**). Permeability value after the long-term test is slightly  
440 higher than the value at the short-term 90 °C. It indicates that the internal fracture aperture is  
441 deformed with time in the long-term. The irreversible crushing and/or dissolution at the  
442 contacting asperities, rather than elastic and reversible compaction, might be a reason for the  
443 phenomena in the long-term tests (Yasuhara, et al., 2015).



444 Fig. 15 Summary of permeability changes in short- and long-term tests

445

#### 447 4.2 Relation between hydraulic aperture and mechanical aperture

448 **Figure 16(a)** shows a comparison of the mechanical aperture measured by the CT  
449 observation and the hydraulic aperture which is calculated from the mechanical aperture. The  
450 mechanical aperture shows a relationship between the hydraulic aperture and the fracture  
451 contact-area ratio, as defined by, Farough et al. (2016).

$$452 \quad b_H^3 = b_M^3(1-R_C)/(1+R_C) \quad (7)$$

453 where  $b_H$  is the hydraulic aperture,  $b_M$  is mechanical aperture measured from CT observation,

454 and  $R_C$  is the fracture contact-area ratio calculated from CT image analysis. It is noted from **Fig.**  
455 **16(a)** that hydraulic aperture is lower than the mechanical aperture, which is congruent with the  
456 previous studies that the real roughness characteristics influence the fluid passing through the  
457 aperture, and result in the hydraulic aperture lower than the mechanical aperture (Bandis et al.,  
458 1985; Renshaw,1995; Zimmerman et al.,2004; Liu, 2005;Kishida et al., 2013).

459

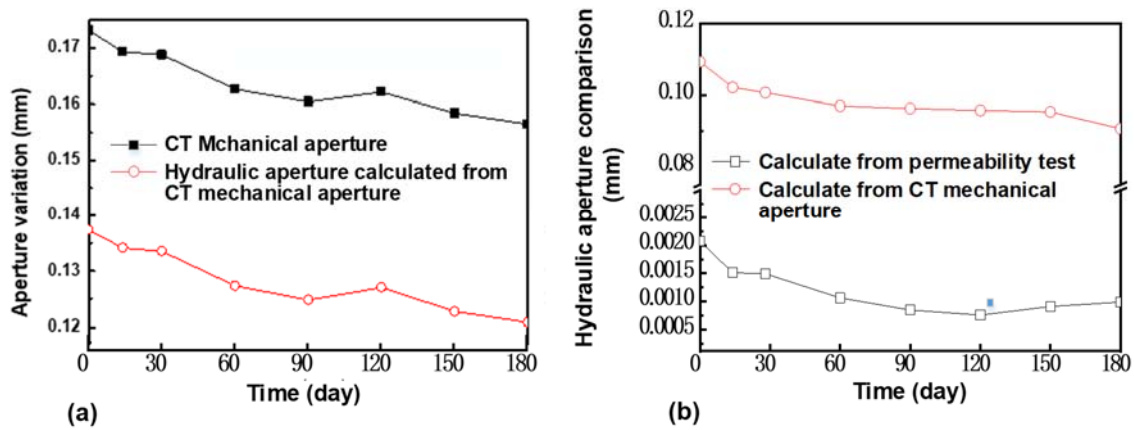
#### 460 **4.3 Relation between permeability and internal fracture structure**

461 To investigate the correlation between variations in the permeability and the fracture  
462 aperture, the relation between hydraulic aperture measured from the permeability tests and that  
463 calculated from the X-ray CT observation of the mechanical aperture is discussed. **Figure 16(b)**  
464 shows a comparison of the hydraulic apertures. The hydraulic aperture obtained from the  
465 permeability test is always lower than that obtained from the X-ray CT. From the initial  
466 hydraulic aperture value to the end value after 180 days, the decreasing gradient of the hydraulic  
467 aperture obtained from the permeability tests is about 52 % and the decreasing gradient from  
468 the CT is about 12 %.

469 The difference in the decreasing gradient mentioned above will be summarized for two  
470 reasons. Firstly, the granite Sample #1 experiences various levels of temperature and confining  
471 pressure in the short-term tests before the long-term tests, whereas the granite Sample #2 is  
472 used only for the long-term permeability tests. The fracture roughness surfaces of Sample #1  
473 are deformed during the short-term tests under the coupled conditions. This deformation results  
474 in the evolution of the aperture and the decrease of the initial aperture value. Secondly, the two  
475 samples are different in size, which should also generate the initial aperture difference.

476 However, from **Fig.16(b)**, it is noted that the hydraulic aperture evolutions obtained by the

477 permeability tests and X-ray CT are quite similar. Both of them show that the hydraulic aperture  
 478 decreases with time. From this comparison, the reason for the permeability decreasing in the  
 479 long-term can be explained by the temporal change in the fracture aperture.



480  
 481 **Fig. 16** (a) Comparison result of CT mechanical aperture and hydraulic aperture. (b) Comparison of changes in hydraulic  
 482 aperture calculated from permeability tests and from CT observation of mechanical apertures  
 483

## 484 5 Conclusions

485 This study carried out a series of short- and long-term permeability tests under various  
 486 conditions. To clarify the long-term permeability evolution, variations in the fracture aperture  
 487 and fracture contact-area were investigated using a microfocus X-ray CT and image analysis  
 488 technique. The relation between the hydraulic aperture obtained from the permeability tests and  
 489 that estimated from the X-ray CT observation was evaluated. The influence of the internal  
 490 fracture structure variation on the permeability was discussed. The conclusions are drawn as  
 491 follows:

- 492
- 493 1) In the short-term permeability tests, permeability is sensitive to the confining pressures and  
 494 temperatures and shows a clear temperature-dependence - it drastically decreases when  
 495 applying a high temperature of 90 °C. The internal fracture may be reversibly deformed,  
 496 and permeability is not sensitive to the confining pressure when reloading the confining

497 pressure again before starting the long-term tests. In the long-term tests, permeability  
498 evolution showed that the internal fracture structure was irreversibly deformed under a  
499 long-term compaction condition. This deformation will change permeability behavior.

500 2) Triaxial apparatus coupled with the microfocus X-ray CT equipment was developed in this  
501 study, to observe the long-term fracture structure change under various levels of confining  
502 pressures and temperatures. Results show that the fracture aperture decreases with increase  
503 of the confining pressure from 0 MPa to 3 MPa, and it gradually decreases with time at the  
504 constant confining pressure of 3MPa during 180 days.

505 3) The morphology of the fracture surface roughness was successfully obtained by the CT  
506 image analysis, which showed quite similar to that was obtained by the optical profilometer.  
507 The fracture contact-area ratios calculated by the image analysis are relatively larger than  
508 those shown in the literature. However, this study successfully obtains the contact-area ratio  
509 through the experimental approach and captures the long-term evolution of an actual rock  
510 fracture structure, i.e., the increase in the fracture contact area with time under the constant  
511 confining pressure.

512 4) The hydraulic aperture value measured from the long-term permeability tests is lower than  
513 that estimated from the X-ray CT. This mismatch is probably attributed to the difference of  
514 the sample size. Moreover, before the long-term permeability tests, Sample #1 experienced  
515 short-term permeability tests, and the aperture distribution has been altered under the  
516 coupled conditions, while Sample #2 was not used to conduct any pre-tests. Those reasons  
517 may lead to the difference in the hydraulic aperture values.

518 5) The decreasing gradient of the hydraulic aperture measured from the permeability tests is  
519 qualitatively similar to that estimated from the CT observation. Therefore, the fracture

520 contact-area increase under the constant confining pressure links to the permeability  
521 decrease in the long-term.

522

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527

528

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