- Shallow Subsurface Structure in the Hualien Basin
- and Relevance to the Damage Pattern and Fault
- Rupture During the 2018 Hualien Earthquake

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- 4 Abstract. The 2018 Hualien earthquake (Mw6.4) generated a large peak-
- 5 to-peak velocity of over 2 m/s with a period of 3 s at the south end of the
- 6 Milun fault, which resulted in the collapse of five buildings. To investigate
- the shallow subsurface soil structure and evaluate possible effects on the ground
- 8 motion and building damage, we performed microtremor measurements in
- 9 the Hualien basin. Based on the velocity structure jointly inverted from both
- Rayleigh-wave dispersion curves and microtremor Horizontal-to-Vertical (H/V)
- spectral ratio data, we found that the shallow subsurface structure gener-
- ally deepens from west to east. Close to the Milun fault, the structure be-

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- comes shallower which is consistent with faulting during the 2018 earthquake
- and the long-term tectonic displacement. There is no significant variation
- 15 for the site conditions in the north-south direction that can explain the large
- peak ground velocity in the south. As a result of the dense measurements
- in the heavily damaged area, where three high-rise buildings totally collapsed,
- these locations have the AVS30 values (average S-wave velocity of the up-
- per 30 m) are relatively high compared to the more distant area from the
- ₂₀ Meilun river. This is somewhat unusual since lower AVS30 values indicat-
- ing softer ground conditions are expected close to the river. We did not find
- 22 any characteristic subsurface soil structure which may contribute to the build-
- ing collapses. The large 3 s pulse was probably generated by source effects,
- rather than subsurface soil amplification.

Introduction

- The 2018 Hualien earthquake in Taiwan (Mw 6.4, at 23:50:43, February 6, 2018, local
- 26 time) showed a very complex fault structure. The moment tensor mechanism shows a
- ²⁷ substantial non-double couple component (e.g. USGS website, see Data and Resources
- Section), which suggests there were multiple fault geometries. The source models in the
- seismic waveform and geodetic inversions [e.g. Lee et al., 2019; Huang and Huang, 2018;
- Lo et al., 2019 use multiple fault planes to explain the observed data.
- The Milun fault, one of the fault structures causing the earthquake, runs in a north-
- south (NS) direction through the center of the Hualien basin (Figure 1). This fault
- previously ruptured on October 22, 1951, causing a $M_L7.1-7.3$ earthquake [Lo et al., 2012].
- At that time, surface rupture appeared in downtown Hualien [Huang et al., 2019] from the
- Qixingtan coast, through the west side of Meilun Mountain, to the old port (see Data and
- Resources Section). This fault was likely reactivated during the 2018 Hualien earthquake
- ³⁷ [Huang et al., 2019; Lin et al., 2019; Wu et al., 2019a]. Source models suggest that the
- fault dips to the east, and the slip is thrust movement with a left-lateral component [Lee
- et al., 2019; Kuo-Chen et al., 2019; Lo et al., 2019].
- There was an unusual pattern in the damage of the high-rise buildings. In Hualien city,
- four buildings totally collapsed with story failure and one totally collapsed without story
- 42 failure. All of these structures were located very close to the Milun fault based on the
- Reconnaissance report by the National Center for Research on Earthquake Engineering
- (NCREE) (see Data and Resources Section). Researchers have debated the relationship
- between the observed damage distribution and the fault rupture [e.g. Huang et al., 2019;
- 46 Lin et al., 2019].
- Ground motions at a site are influenced by the source, travel path, and local site char-
- acteristics. One possible explanation is that the building damage resulted from ground

- motion amplification due to local soil structure. In this study, we performed microtremor
 measurements to investigate the shallow subsurface soil structure in the Hualien basin.
 We set a measurement line along a northwest-southeast section of the Hualien basin across
 the fault to see the difference in the shallow velocity structure. We also made measurements in the heavily damaged area where three high-rise buildings collapsed. Based on
 the inverted subsurface velocity structure, we will discuss the relationship between the
 - Strong Motion and Building Damage

subsurface soil structure and building damage.

- The strong motions during the 2018 Hualien earthquake were recorded by the dense seismic networks of the Central Weather Bureau (CWB) in Taiwan [Shin et al., 2013] and the P-Alert Strong Motion Network [Wu et al., 2019b]. Downtown Hualien is located in a narrow basin (width of several kilometers) between the Central Mountain Range and the Pacific Ocean (Figure 1). The Milun fault runs in a NS direction through the center of the Hualien basin. Geology of the west side of the Milun fault is alluvium, and east side of the fault consists of either conglomerate or sandy layer.
- There are 20 stations in the Hualien basin with average spacing of about 1 km. Figure 2 shows the velocity records at the strong motion stations on the east and west sides of the fault from north to south. The locations of the seismic stations are shown in Figure 3.

 The main pulse has a period of 3 s, and the phases of the waveforms are rather different between the east and west sides of the fault for the NS component. The arrival of this large pulse is about 5 s later than the S-wave arrival from the hypocenter, which suggests the source of this pulse is away from the hypocenter.
- The acceleration response spectra in Figure 4 show different spatial patterns depending on the period. The distribution of the response spectra at 0.5 s is relatively homogeneous

over the basin, and the stations on the western side of the basin (HWA048 and HWA028)
show slightly higher values. This suggests that the western side of the basin consists of
thinner deposits, which may amplify the shorter period ground motion. On the other
hand, the long-period ground motions with periods of 2-3 s were strongly amplified near
the southern end of the Milun fault.

A damage survey of the high-rise buildings was carried out in the Hualien basin by
Kuo et al. [2018]. Note that the definition of high-rise buildings in Taiwan is 10 or more
floors. There are five buildings rated as damage rank 5 according to the damage scale
of Hsiao et al. [1999], i.e., complete destruction, but most of the high-rise buildings were
undamaged or sustained minor damage [Kuo et al., 2018]. The locations of the heavily
damaged buildings are shown in Figure 3. It is interesting that the heavily damaged
buildings are all very close to the fault surface rupture, but not concentrated near the
southern end of the Milun fault, where the large peak ground velocity was recorded
(around the station W028).

Microtremor Survey

- We performed microtremor surveys in the Hualien basin from October 20 to 26, 2018.
- ⁸⁷ We used ten seismometers (JU410) made by Hakusan Corporation to perform array mea-
- surements. The JU410 instrument includes 3 component acceleration-type sensors, a
- 89 logger, and a battery, in casing. The sampling frequency was set to 200 Hz with the
- ₉₀ high-cut filter set at 80 Hz.
- We performed small (scale of about 10 m) and large (scale of a few hundred meters)
- ⁹² array measurements. The small array measurements were performed with 5 seismometers
- ₉₃ in arrays consisting of a regular triangle with a radius of 0.6 m, and two seismometers
- set further apart along the line of the center of the triangle (see Figure 5(d)). The

distance of the two seismometers from the triangle is about 10 and 15 m. We performed
these array measurements at 64 locations shown in Figure 3. Locations of the small
arrays were selected for three purposes. First, we measured along the line X-Y with a
spacing of 50–200 m to obtain an east-west (EW) profile of the Hualien basin. We also
performed 22 measurements within the heavily damaged area D in Figure 3 to evaluate the
effect of subsurface soil structure on the building damage. For calibration, we performed
measurements at the 7 strong motion stations [Kuo et al., 2012] and the marble factory
(MF) [Okamoto et al., 1998] where borehole logging data are available. We performed
measurements for 15 minutes at each location.

Large array measurements were performed at two locations, on the east and west sides
of the Milun fault (arrays E and W in Figure 3). At each site, three different size array
measurements (maximum radii of 100, 300, and 600 m) were performed. Each measurement was performed with 7 seismometers; one at the center, three at the corners of a
regular triangle, and three at the corners of the medial triangle. The array geometries are
shown as solid triangles in Figure 3. The duration of the measurement is 45 minutes.

Small array measurements were also performed at each center point of the large arrays to obtain subsurface velocity models for a wide depth range. In addition to this, medium size array measurements (radii of 9 and 17 m) were conducted by using either regular or irregular triangle arrays with three seismometers so that we can complementarily check the analysis results for both the small and large arrays.

The acceleration sensor in the instrument we used achieved a low noise level by optimizing the active element circuit [Tomioka and Yamamoto, 2006]. According to the
specification, the noise level is less than 0.1 $[\mu G/\sqrt{Hz}]$ at 1-30 Hz and it was below this
level at 0.5-40 Hz in the performance test [Tomioka and Yamamoto, 2006]. We confirmed
that the H/V spectrum obtained by our measurement showed a good agreement with

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that obtained by a broadband velocity sensor at the frequency 0.2-20 Hz [Kuo et al., 2019] (Figure S1).

Method

The obtained microtremor data were processed with the following methods.

123 H/V Spectral Ratios

The H/V spectral ratios [Nakamura, 1989] at each observation point were computed 124 from the three-component microtremor waveforms. First, we split the time series into 125 windows of 4096 points (20.48 s), with a 50% overlap; this resulted in approximately 50 windows for each measurement. This window length should be sufficient to capture 127 low-frequency information for H/V in the range of 0.1–0.5 Hz. Before transforming the 128 time windows into the frequency domain, a weighted Hanning window was applied. Win-129 dows with obvious transient noise were excluded from the analysis. A Fast Fourier Tran-130 form (FFT) was applied to each individual time window to obtain the Fourier amplitude 131 spectrum. Those spectra were then smoothed by a Konno-Ohmachi filter [Konno and 132 Ohmachi, 1998] with a smoothing coefficient value b=20. The horizontal component is defined as the geometric mean of the two components [Bard et al., 2008]. We visually checked that the peak frequencies of the two components were very similar. We used five seismometers at each observation point, and consequently, we averaged the five H/V spectral ratios. We resampled the H/V curves with 64 logarithmically spaced samples 137 between 0.25 and 10 Hz. These resampled curves were used as input to the inversion 138 analysis. 139

140 Phase Velocity

In order to obtain the Rayleigh-wave phase velocities, we applied the spatial autocorrelation (SPAC) method [Aki, 1957] to the vertical-component microtremor array data.

In the determination of the phase velocities, power and cross spectral densities were estimated with the techniques of both smoothing and ensemble averaging in the frequency domain [Bendat and Piersol, 2010]. The waveforms of each small array were split into windows of 10.24 s duration with 50% overlap, this resulted in approximately 100 windows per site, and a weighted Hanning window was applied. We apply a Fast Fourier Transform (FFT) to obtain magnitude-squared FFT spectra, which were then smoothed using a Parzen window with a bandwidth of 0.3 Hz. The smoothed spectra were averaged at each frequency (i.e., ensemble average).

A shorter window length was used to process the microtremor array data than for H/V spectral analysis because the focus was on frequencies greater than a few hertz. It also enables stacking a large number of data segments, which contributes to improving the robustness. A phase-velocity dispersion curve may exhibit abrupt changes in frequencies higher than 10 Hz at a site with thin sedimentary layers. Without a priori information on the local site condition, frequency-dependent windowing sometimes causes over smoothing in high frequency. Therefore, we used the Parzen window with a bandwidth of 0.3 Hz to avoid over-smoothing at higher frequencies.

The calculated spectral densities were used to calculate the real part of the complex coherencies (SPAC coefficients) The obtained Rayleigh-wave phase velocities were resampled with logarithmically spaced samples between a few (1.1–3.0 Hz depending on sites) to 20 Hz and used for the subsequent inversion analysis.

Joint Inversion for the S-wave Velocity Structure

We inverted for the S-wave velocity (Vs) structure using the Rayleigh-wave phase velocities and H/V spectral ratios following the method of Arai and Tokimatsu [2005]. First,
we constructed the initial model from the PS logging data at the surrounding strong motion stations (see Data and Resources Section). The logging data at the stations west of

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the fault consist of three major layers: 1) very silty or clayey sand (Vs~200m/s), 2) silty gravels or well-graded gravels (Vs~300m/s) and 3) silty sand or silts with very fine sand $(V_s \sim 350 \text{m/s})$. We used these three layers for the top three layers of the initial model (Table 1). The logging data at the stations east of the fault include a silty gravel layer 171 with higher velocity (Vs~600m/s) which we used as a fourth layer of the initial model. 172 We obtained a one-dimensional velocity structure model at each observation point by 173 iteratively improving the above initial model to explain the observed phase velocities and 174 H/V spectral ratios. During the inversion procedure, the thickness and the S-wave velocity 175 in each layer were set to be unknown parameters. The density was estimated based on 176 the empirical relationship with the P-wave velocity (Vp) [Gardner et al., 1974] and Vp 177

Since the observed H/V spectral ratios have multiple peaks, we considered single modes 179 and multiple modes for both the Rayleigh and Love waves in the inversion procedure, 180 where the power partition ratios of Rayleigh to Love waves (R/L) were fixed to 0.7, as 181 suggested by Arai and Tokimatsu [2005]. Another approach to reducing the number of parameters is to use a fixed ratio of horizontal to vertical loading forces (HVLF) [Picozzi et al., 2005; Parolai et al., 2005]. Both the fixed R/L and the fixed HVLF are techniques for the simplification to compute the theoretical H/V spectra. We used a fixed R/L which was observed from the field data and stable over time [e.g. Arai and Tokimatsu, 2000]. The weights on the H/V spectral ratio and the phase velocity dispersion curve for the inversion 187 were set to 0.2 and 0.8, respectively. The weight of the H/V spectral ratio is small, but adding them increases the resolution at depth. A search range for the S-wave velocity in 189 each layer was limited to 20% from the initial model, while no constraint was imposed 190 on the thickness. The analysis was done by using an analysis code "TremorDataView" 191 [Senna and Fujiwara, 2008]. 192

was fixed at the initial model.

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At the large array sites, following Foti et al. [2018], the maximum depths of investigation were assumed to be the maximum aperture of the arrays or less (i.e., several hundreds of meters). At the small array sites, on the other hand, the maximum depths of investigation were assumed to be several tens of meters, or a few times larger than the maximum array aperture. This expectation is based on our experience that small arrays have better relative resolution as compared to large arrays. As well, a joint analysis of phase velocity and H/V spectral data seems more effective for smaller arrays from the perspective of extending the analysis to low-frequency ranges.

201 Analysis of Large Array Data

It was difficult to construct a detailed initial model to the depths corresponding to the large array surveys, due to the lack of data constraining geologic/geotechnical parameters at depth. Therefore, the large array data were analyzed by a method similar to that for the small arrays with the following difference. The duration and number of data segments used for the ensemble average were 20.48 s and 92 or 40.96 s and 53, respectively, depending on the array size. The bandwidth of the Parzen window was set to 0.1 or 0.3 Hz. We selected these values to avoid over smoothing of the spectra at the target frequency. The phase velocity in the low frequency (<2Hz) domain was obtained by reading zero-crossing points of the SPAC coefficients [Ekström et al., 2009].

Unlike the small arrays which have relatively more information on the shallow structure,
the information to the depths corresponding to the large array is limited. Therefore, we
constructed an initial model empirically [Ballard Jr, 1964]. The initial models (number
of layers and Vs) is updated by an empirical Bayesian approach [Cho and Iwata, 2019] to
better explain the phase velocity dispersion curve. It enables flexible modeling of shallowto-deep structure by automatically determining the number of layers based on the Bayes

factor. We inverted only the S-wave velocities for multiple thin layers, with the thickness of each layer fixed to a specific value.

Results

$_{\scriptscriptstyle 219}$ H/V~Spectral~Ratios

Figure 5(a) shows the peak frequencies and peak amplitudes of the H/V spectra. The 220 results reflect the local heterogeneous velocity structure, on a macroscopic scale, with a 221 higher frequency peak (about 2 Hz) on the western mountain side (e.g., around the station 222 HWA048), and a lower frequency peak (about 1 Hz) around the Meilun river delta. The 223 east side of the Milun fault, which is close to the coast (e.g., around the station HWA009), 224 is at a slightly higher altitude and the peak frequency is higher than the river sediment 225 area (e.g., around the station HWA019). Figure 6(a) shows the H/V spectra for the EW section along the X-Y line in Figure 3. 227 The peak frequency is higher on the west side of the basin (at 121.58° about 2 Hz), and gradually decreases to the east (at 121.59° about 1 Hz). The spectra at the floodplain of the Milun river (121.605°-121.61°) have a very large amplitude peak at a frequency of 1 Hz, and the amplitude at higher frequencies is very small (Figure 5(a)). This may 231 indicate a strong velocity contrast in the subsurface structure. The east side of the Milun fault shows relatively flat spectra (121.612°–121.615°).

Phase Velocity

We obtained four phase velocity dispersion curves from the different sensor spacings in the small array measurement: a regular triangle with a radius of 0.6 m and pairs of sensors with the distances of about 5 m, 10 m, and 15 m. These curves were connected to obtain a single phase velocity curve across the frequency range of our interest (i.e., a few to 20 Hz). An example of the phase velocity curves at the station HWA011 is shown in Figure 7(b).

Figure 5(b) shows the distribution of the minimum phase velocity of the dispersion curve, which generally corresponds to the S-wave velocity of the shallowest layer. The east side of the Milun fault and west of the railway, clearly shows higher S-wave velocity, at about 250 m/s. The S-wave velocity is lower on the west side of the Milun fault at about 150-200 m/s, probably due to the deposits of the Meilun river.

Figure 5(c) shows the distribution of the AVS30 determined by directly reading the
Rayleigh-wave phase velocity, corresponding to the wavelength of 40 m. It is well known
that the phase velocity at the wavelength of 40 m is a good approximation of AVS30
[Brown et al., 2000; Konno and Kataoka, 2000; Martin and Diehl, 2004; Cho et al., 2008;
Albarello and Gargani, 2010]. The figure indicates that AVS30 values east of the fault are
greater than 300 m/s, whereas west of the fault the values are mostly smaller than 300
m/s.

Figure 8 shows the phase velocity curves, including relatively low frequencies obtained from measurements of the large arrays on the east and west sides of the fault. The phase velocity curves for the two sides of the fault are quite different in the frequency range at 1–10 Hz, indicating that the S-wave velocity of the shallow layers is greater on the east side of the fault compared to the west side of the fault. On the other hand, there may be little difference in the deeper structure.

259 Inverted Velocity Structure

We inverted for the velocity structure from the obtained phase velocity curves. An example of the data fitting at the HWA011 station is shown in Figure 7. The black and gray curves show the observed and calculated data based on the optimal velocity structure, respectively. The fits for both H/V spectra and phase velocity curves are reasonably good.

By inverting those two quantities simultaneously, we were able to obtain the velocity structure to the depth corresponding to the 1 Hz peak of H/V spectrum (about 50–75 m assuming Vs 200–300 m/s). We visually checked the fit of all other sites and confirmed that the velocity models explained the observed data.

268 Hualien Basin Profile

Figure 6(b) shows the velocity structure of the EW section along the X-Y line in Figure
3. There is a large difference between the east and west sides of the Milun fault. The
thickness of the first and second layers (Vs < 300 m/s) gradually increases from west to
east, but suddenly decreases at the location of the fault. This change is much larger than
the change of the topography at the ground surface. There is not a large difference in the
thickness of the first layer, but Vs is very low (< 200 m/s) on the west side of the fault,
which is assumed to be a floodplain of the Meilun river.

276 Deep Structures

Figure 8(b) shows the inverted velocity structure for the phase velocity curves obtained
from the large array measurement. The S-wave velocity of the upper layers (depth < 500
m) is well resolved and greater on array E than on the array W. The greater Vs east of
the fault is consistent with the Hualien basin profile shown in Figure 6(b). The deeper
structure (depth > 500 m) does not seem to have a large difference between the two
arrays.

283 PS logging data at the Strong Motion Stations

To evaluate the accuracy of the velocity estimation, we compared the estimated velocity structure with the borehole PS logging data at the strong motion stations (Figure 9). We have 7 stations with shallow velocity profile logging data (about 30 m depth, see Data and Resources Section) and 1 station with deep logging data to 200 m [Okamoto et al., 1998]. Our results demonstrate good agreement between the obtained Vs depth profile

and the available logging data, except for the HW019 station, where logging data indicate $_{290}$ Vs> 600 m/s at 15 m, whereas the inverted structure shows a Vs of only 350 m/s at the $_{291}$ same depth.

292 Phase velocity curves estimated from the triangle array and linear arrays

In order to verify the reliability of the linear array measurements, we compared phase 293 velocity curves obtained from the triangle and linear arrays at the site of the large array 294 W, where we have triangle arrays with radii of 0.6, 9, and 17 m and 2-point linear arrays 295 with distances of 5, 10, and 15 m. Figure 10(a) shows the phase velocity curves estimated 296 from these arrays. The phase velocity curves estimated from the linear arrays are within 297 ±20% of those estimated from triangle array results. At each site with a small array, we 298 used a triangle array, together with linear arrays, so that we can verify the reliability of 299 the linear arrays at high frequency. For example, as demonstrated in Figures 10(b) and 300 10(c), the results at the strong motion stations show good agreement between the phase 301 velocity curves estimated from the linear and triangle arrays. These results suggest that 302 the wavefield is close to "isotropic", in the sense that it is appropriate to use the SPAC method at these sites.

It is true that an isotropic wavefield is preferable for the SPAC analysis, in particular, 305 when we use a linear array with 2 sensors (2-point array). However, it does not mean that 306 a completely isotropic field is needed to obtain the dispersion curve. A two-point array has larger error than a circular array, but it has the advantage of requiring less space and fewer 308 sensors. Cho [2020] demonstrated that the error is critical if the microtremor wavefield is 300 oriented at a single direction perpendicular to the axis of a 2-point array, but the error 310 becomes smaller if the azimuthal spreading of the wavefield becomes wider. In the field, 311 the assumption of a wavefield with azimuthal spreading is more realistic than assuming 312 a wavefield oriented in a single direction. In fact, Cho [2020] analyzed 400 microtremor 313

array measurements and revealed that most of the 2-point arrays analyzed had an error of <20%. The phase velocity curves in Figure 10 suggest that the effect of an anisotropic wavefield was relatively small in at least the frequency range of these arrays.

Note that the 2-point array may not be suitable for certain situations. For example,
we cannot use the 2-point array for a wavefield with strong directional components (e.g.,
vicinity of factories which produce strong seismic noise). The regular polygon array is
always preferable as long as there is enough space and equipment. When we cannot avoid
using 2-point arrays, we need to check the isotropy of the wavefield for the SPAC analysis.

Discussion

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Subsurface velocity structure and regional tectonics

The Hualien basin is associated with the collision of the Philippine Sea plate and Eurasian plate [Angelier, 1986; Yu et al., 1997]. The basin is long and narrow in the NS direction. The east side of the Milun fault (Hualien tableland) has a higher altitude than the west side of the fault, and the Meilun river runs along the fault (Figure 3).

Although there is a general deepening of the shallow structure from west to east, our results show a large difference in the opposite sense across the Milun fault. The section profile of the velocity structure close to the fault shows that the thickness of the shallow layer is greater on the west side of the fault than on the east side of the fault. The velocity at the depths of less than 150 m, estimated from the large array, is also consistent with this feature. The AVS30 shown in Figure 5(c) also has a strong contrast with lower values on the west side of the fault, and velocities larger than 300 m/s on the east side of the fault.

This velocity difference on the two sides of the fault is consistent with dip-slip faulting due to the tectonic structure [Angelier, 1986; Shyu et al., 2016]. The Hualien tableland was

uplifted during the mainshock [Lee et al., 2019; Huang and Huang, 2018; Lo et al., 2019]. Such uplift might accumulate on the east side of the fault over numerous earthquakes, which results in the higher altitude. The west side of the fault becomes relatively lower,

and sedimentary deposits form the low S-wave velocity layers near the surface.

Note that Figure 6(b) was estimated from the surface wave data, and the heterogeneous 341 structure in the horizontal direction is affected by the resolution depending on the wave-342 length. That is, since the deeper part of the figure was estimated by waves with longer 343 wavelengths, it may have a limited resolution to capture the sharp change of the velocity 344 structure in the horizontal direction.

Relationship to the Pulse-like Strong Motions 346

There was a characteristic pattern in the strong motion distribution in the Hualien 347 basin. The velocity waveforms show a large pulse-like waveform with a period of 3 s (Figure 2) and large amplitudes at the southern end of the Milun fault (Figure 4(d)). 349 This was observed on both the eastern and western sides of the fault. Ground motions are influenced by the source, path, and site characteristics. One possible explanation is the large velocity pulse with 3 s period was generated by the local site response.

The results of our survey show that there is no significant shallow subsurface difference at the southern end of the Milun fault in comparison to the northern end, which could 354 explain the distribution of building damage in this region. Figure 11 shows the S-wave velocity structure in the NS direction along the Meilun river (along the Z-Z' section in 356 Figure 3). The section shows a horizontally layered structure and no significant change 357 along the Milun fault. This is consistent with the tectonic regime of the Hualien region. 358 Due to the EW compressional tectonics, there is a substantial change of velocity structure 350 in the EW direction (Figure 6(b)), but little variation in the NS direction (Figure 11).

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Based on our large array measurements, Vs reached 750 m/s at a hundred meter depth.

Suppose the 3 s velocity pulse was the response of the local velocity structure, then

we would need a strong velocity contrast with a thick low-velocity deposit (e.g. 450 m

thickness assuming Vs 600 m/s). Figure S2 shows the transfer functions for the velocity

structures estimated from the large array measurements based on the one-dimensional

elastic site response [Haskell, 1960]. The predominant frequencies for the array E and W

are about 0.8 and 0.5 Hz, respectively.

The peak period of the ground motion during the mainshock was 3 s, but our data 368 showed that it was difficult to explain this period from the subsurface soil amplification at 369 least for the linear response. Figure 2 shows the pulse-like ground motions are commonly 370 observed at most stations, but the phase seems to be different on the east and west sides 371 of the fault. The displacement records after the integration of these data show the static 372 offset at this time [Kuo et al., 2019]. Kuo et al. [2019] concluded that this pulse-like 373 ground motion might have been caused by the asperity, forward directivity amplification, 374 and radiation pattern rather than the local site effect. Other studies also explain this 3-s pulse by source effects, such as rupture directivity and near-field waveform from the shallow fault segment with a large slip [Wen et al., 2019; Miyakoshi et al., 2019]. Therefore, although we cannot exclude the possibility of the non-linear response of the subsurface soil 378 structure or 2D/3D basin effects [Kawase, 1996], our results suggest that the 3 s velocity pulse was more likely generated by a source effect, rather than the local site response. 380

Relationship to the Building Damage

There were five buildings which were completely destroyed during the mainshock, and all of them were located very close to the fault surface rupture. It might be expected that the large velocities with 3 s period at the southern end of the Milun fault might be responsible for the damage to high-rise buildings, but the spatial pattern of long-period

ground motions does not match the overall distribution of collapsed buildings (Figure 4(d)). We focused on the heavily damaged area D in Figure 3, where three buildings collapsed, and performed dense microtremor measurements to investigate the possible effect of local site characteristics on the damage of the structures.

Kuo et al. [2018] performed a damage survey for the high-rise buildings with 10 or

more stories in that area. As shown in Figure 12, the buildings close to the river have 391 more severe damage. Therefore, there is a debate on whether the reason for the collapsed 392 buildings is the subsurface amplification due to the deposits of the river. The AVS30 393 distribution obtained from our survey showed slightly higher values close to the river 394 (Figure 12). This suggests that the shallow layers close to the river are unexpectedly hard 395 compared to those farther from the river. This is probably due to the dip-slip faulting, as 396 we have seen in Figure 6(b). The first and second layers with low Vs have become thinner 397 on the east side compared to the west side of the fault because of the vertical deformation. 398 The natural period of the reinforced concrete structure can be approximated by 0.07N 399 (where N is the number of the floors) [Hong and Hwang, 2000]. We also performed microtremor measurements at the two 13-floor buildings, and their natural periods were 0.5 s and 0.9 s, respectively. Wang et al. [2018] also estimated the natural period of highrise buildings as 0.34–0.65 s from their microtremor survey. The design spectra for these 403 periods are much higher than observed ground motions [Wang et al., 2018]. Therefore, high-rise buildings that satisfy the building code should not be seriously damaged by the 405 ground motion corresponding to the linear site response (about 1 Hz). On the other hand, 406 the ground motions at the period 2-3 s are extremely large and exceed the design level. 407 There are various possibilities for the cause of the collapse of the buildings, such as 408

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construction deficiencies (e.g. antiquate building codes, soft story and rooftop additions

indicated by [Lin et al., 2020a]), static offset at the fault, near-source ground motion. If

buildings do not have enough seismic capacity, damage caused by a moderate shaking can
cause severe degradation, which significantly increases the natural period of the building
during the shaking. To understand the cause of building collapse, the site specific ground
motion estimation and structure response analysis are necessary. However, from our field
survey, the linear site response was dominant near 1 Hz, which did not explain the large
pulse exceeding the design level.

Conclusions

- We performed microtremor measurements in the Hualien basin in order to investigate
- the shallow subsurface soil structure and evaluate their effects on the ground motion and
- building damage during 2018 Hualien earthquake. We have three major conclusions which
- may contribute to the clarification of the large velocity pulse and building damage.
- 1) Based on the inverted subsurface velocity structure, we found that the shallow subsur-
- face structure deepens from west to east and then becomes shallower at the Milun fault.
- The shallowing across the fault is consistent with the faulting during the mainshock and
- the long-term tectonic displacement. Due to this offset structure across the fault, the
- AVS30 of the west side of the fault is generally smaller than that of the east side of the
- 426 fault.
- 2) Our survey results show that there is no significant difference in the shallow structure
- at the southern end of the Milun fault, where very large peak-to-peak velocity over 2 m/s
- was recorded. This large amplitude 3 s pulse was probably generated by a source effect,
- rather than subsurface soil amplification.
- 3) As a result of the dense measurements in the damaged area, the locations where three
- buildings totally collapsed had relatively large AVS30 values compared to the areas farther
- from the Meilun river. This suggests that the subsurface soil structure close to the river

is unexpectedly harder compared to farther from the river. To clarify the cause of the collapse of these buildings, we need further investigations on the building construction and earthquake source characteristics.

Data and Resources

We used the seismic waveform data recorded by the CWB and the P-Alert Strong Mo-437 tion Network. The data can be obtained from the website at https://gdms.cwb.gov.tw/ and https://palert.earth.sinica.edu.tw/index_e.php. The moment tensor mech-430 anism of the 2018 Hualien earthquake is available at the USGS website (https:// 440 earthquake.usgs.gov/earthquakes/eventpage/us1000chhc/executive). The PS log-441 ging data at the strong motion stations are available at Engineering Geological Database 442 for TSMIP (http://egdt.ncree.org.tw/HWA_eng.htm). 443 The fault map in Hualien was obtained from: Hualien Prefecture Eastern Region En-444 vironmental Geology Research (http://geo.cpami.gov.tw/Case/97%E8%8A%B1%E8%93% AE%E7%B8%A3%E8%8F%AF%E6%9D%B1%E5%9C%B0%E5%8D%80%E7%92%B0%E5%A2%83%E5%9C%B0% E8%B3%AA%E7%A0%94%E7%A9%B6.htm, in Chinese). The geology map in Hualien was obtained from the National Geological Data Warehouse (https://gis3.moeacgs.gov. tw/gwh/gsb97-1/sys8/t3/index1.cfm, this link is no longer available). sance report of seismic damages provided by the NCREE (in Chinese) is available at (https://www.ncree.org/EarthquakeInfo/20180206/NCREE-2018-005F%E5% 451 8B%98%E7%81%BD%E5%A0%B1%E5%91%8A.pdf). 452 We used an analysis code "TremorDataView" [Senna and Fujiwara, 2008] for the joint 453

We used an analysis code "TremorDataView" [Senna and Fujiwara, 2008] for the joint inversion of velocity structures. The code used to determine observed phase velocities was a modified version of Cho et al. [2008]. The code is available at https://staff.aist.

go.jp/ikuo-chou/bidodl_en.html (last accessed February 2020). Some plots were made

- using the Generic Mapping Tools version 4.5.7 [Wessel and Smith, 1991]. All websites were
- last accessed February 2020.
- We have two Supplemental Figures in the Supplemental Material.

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References

- 464 Aki, K. (1957). Space and time spectra of stationary stochastic waves, with special
- reference to microtremors. Bull. Earthq. Res. Inst. Univ. Tokyo, 35:415–456.
- ⁴⁶⁶ Albarello, D. and Gargani, G. (2010). Providing NEHRP soil classification from the
- direct interpretation of effective Rayleigh-wave dispersion curves. Bull. Seism. Soc.
- Am., 100(6):3284-3294.
- ⁴⁶⁹ Angelier, J. (1986). Preface. *Tectonophys.*, 125(1):IX–X.
- ⁴⁷⁰ Arai, H. and Tokimatsu, K. (2000). Effects of Rayleigh and Love Waves on Microtremor
- 471 H/V Spectra. Proc. of the 12th World Conf. on Earthq. Eng., ref.2232.
- 472 Arai, H. and Tokimatsu, K. (2005). S-wave velocity profiling by joint inversion of mi-
- crotremor dispersion curve and horizontal-to-vertical (H/V) spectrum. Bull. Seism.
- Soc. Am., 95(5):1766–1778.
- Ballard, R. F., Jr. (1964). Determination of soil shear moduli at depths by in-situ vibra-
- tory techniques. *Miscelaneous Paper*, No. 4-691. Vicksburg, MS: U.S. Army Engineer
- Waterways Experiment Station, Corps of Engineers.

- Bard, P.-Y., Acerra, C., Aguacil, G., Anastasiadis, A., Atakan, K., Azzara, R., Basili, R.,
- Bertrand, E., Bettig, B., Blarel, F. et al. (2008). Guidelines for the implementation of
- the H/V spectral ratio technique on ambient vibrations measurements, processing and
- interpretation. Bull. Earthq. Eng., 6:1–2.
- Bendat, J. S. and Piersol, A. G. (2010). Random data: analysis and measurement proce-
- dures. John Wiley & Sons.
- Boore, D. M., Joyner, W. B., and Fumal, T. E. (1993). Estimation of response spectra
- and peak accelerations from western North American earthquakes: An interim report.
- U.S. Geol. Surv. Open-File Rept. 93-509.
- Brown, L., Diehl, J. G., and Nigbor, R. L. (2000). A simplified procedure to measure
- average shear-wave velocity to a depth of 30 meters (VS30). Proceedings of 12th World
- Conf. Earthq. Eng..
- ⁴⁹⁰ Cho, I. (2020). Two-sensor microtremor SPAC method: potential utility of imaginary
- spectrum components. *Geophys. J. Int.*, 220(3):1735–1747.
- 492 Cho, I. and Iwata, T. (2019). A Bayesian approach to microtremor array methods for
- estimating shallow S wave velocity structures: Identifying structural singularities. J.
- 494 Geophys. Res. Solid Earth, 124(1):527–553.
- ⁴⁹⁵ Cho, I., Tada, T., and Shinozaki, Y. (2004). A new method to determine phase velocities
- of Rayleigh waves from microseisms. Geophysics, 69:1535–1551.
- ⁴⁹⁷ Cho, I., Tada, T., and Shinozaki, Y. (2008). A new method of microtremor exploration us-
- ing miniature seismic arrays: quick estimation of average shear velocities of the shallow
- soil. Butsuri-Tansa (Geophysical Exploration), 61:457–468. (in Japanese with English
- abstract).
- ⁵⁰¹ Cho, I., Senna, S., and Fujiwara, H. (2013). Miniature array analysis of microtremors.
- Geophysics, 78(1):KS13-KS23.

- Ekström, G., Abers, G. A., and Webb, S. C. (2009). Determination of surface-wave phase
- velocities across USArray from noise and Aki's spectral formulation. Geophys. Res.
- Lett., 36(18):L18301.
- Foti, S., Hollender, F., Garofalo, F., Albarello, D., Asten, M., Bard, P. Y., Comina, C.,
- ⁵⁰⁷ Cornou, C., Cox, B., Di Giulio et al. (2018). Guidelines for the good practice of surface
- wave analysis: a product of the InterPACIFIC project. Bull. Earthq. Enq., 16(6):2367—
- 509 2420.
- ⁵¹⁰ Fujimoto, K. and Midorikawa, S. (2006). Relationship between average shear-wave veloc-
- ity and site amplification inferred from strong motion records at nearby station pairs.
- Journal of Japan Association for Earthquake Engineering, 6(1):11–22. (in Japanese).
- Gardner, G., Gardner, L., and Gregory, A. (1974). Formation velocity and density the
- diagnostic basics for stratigraphic traps. Geophysics, 39(6):770–780.
- Haskell, N. A. (1960). Crustal reflection of plane SH waves. J. of Geophys. Res. (1896-
- 1977), 65(12):4147–4150.
- Hong, L. and Hwang, W. (2000). Empirical formula for fundamental vibration periods of
- reinforced concrete buildings in Taiwan. Earthq. Eng. and Struct. Dyn., 29(3):327–337.
- Hsiao, C. P., Yeh, H. H., Sheu, M. S., Tsai, K. C., and Ding, Y. Q. (1999). General
- Report on Damage in Chi-Chi Earthquake Damage Investigation for Building Struc-
- tures. NCREE Research Report (No. NCREE-99-054). National Center for Research
- on Earthquake Engineering, Taiwan.
- Huang, M. and Huang, H. (2018). The complexity of the 2018 Mw 6.4 Hualien earthquake
- in east Taiwan. Geophys. Res. Lett., 45(24):13,249–13,257.
- 525 Huang, S.-Y., Yen, J.-Y., Wu, B.-L., Yen, I.-C., and Chuang, R. Y. (2019). Investigating
- the Milun fault: The coseismic surface rupture zone of the $2018/02/06 M_L$ 6.2 Hualien
- earthquake, Taiwan. Terr. Atmos. Ocean. Sci., 30(3):311–335.

- Kawase, H. (1996). The cause of the damage belt in Kobe: "The basin-edge effect," con-
- structive interference of the direct S-wave with the basin-induced diffracted/Rayleigh
- waves. Seismo. Res. Lett., 67(5):25-34.
- Konno, K. and Kataoka, S. (2000). An estimating method for the average S-wave veloc-
- ity of ground from the phase velocity of Rayleigh wave. Trans. Jpn. Soc. Civ. Eng.,
- 2000(647):415-423.
- Konno, K. and Ohmachi, T. (1998). Ground-motion characteristics estimated from spec-
- tral ratio between horizontal and vertical components of microtremor. Bull. Seism. Soc.
- Am., 88(1):228-241.
- 537 Kuo, C.-H., Wen, K.-L., Hsieh, H.-H., Lin, C.-M., Chang, T.-M., and Kuo, K.-W. (2012).
- Site classification and VS30 estimation of free-field TSMIP stations using the logging
- data of egdt. Engineering Geology, 129:68–75.
- Kuo, K., Hsieh, P., Xu, S., and Lin, S. (2018). Preliminary analysis on damage of building
- structures and interior space in the 20180206 Hualien earthquake. The 14th National
- ⁵⁴² Conference on Structural Engineering, Paper No. 24002. (in Chinese).
- 543 Kuo, C.-H., Huang, J.-Y., Lin, C.-M., Hsu, T.-Y., Chao, S.-H., and Wen, K.-L. (2019).
- Strong ground motion and pulse-like velocity observations in the near-fault region of
- the 2018 mw 6.4 hualien, taiwan, earthquake. Seismo. Res. Lett., 90(1):40–50.
- Kuo-Chen, H., Guan, Z., Sun, W., Jhong, P., and Brown, D. (2019). Aftershock sequence
- of the 2018 Mw 6.4 Hualien earthquake in eastern Taiwan from a dense seismic array
- data set. Seismo. Res. Lett., 90(1):60-67.
- Lee, S., Lin, T., Liu, T., and Wong, T. (2019). Fault-to-fault jumping rupture of the 2018
- Mw 6.4 Hualien earthquake in eastern Taiwan. Seismo. Res. Lett., 90(1):30–39.
- Lin, Y.-S., Chuang, R. Y., Yen, J.-Y., Chen, Y.-C., Kuo, Y.-T., Wu, B.-L., Huang, S.-Y.,
- and Yang, C.-J. (2019). Mapping surface breakages of the 2018 Hualien earthquake by

- using UAS photogrammetry. Terr. Atmos. Ocean. Sci., 30(3):351–366.
- Lin, J.-L., Kuo, C.-H., Chang, Y.-W., Chao, S.-H., Li, Y.-A., Shen, W.-C., Yu, C.-H.,
- Yang, C.-Y., Lin, F.-R., Hung, H.-H., et al. (2020). Reconnaissance and learning after
- the February 6, 2018, earthquake in Hualien, Taiwan. Bull. Earthq. Eng., 18:4725–4754.
- 557 Lin, Y.-Y., Kanamori, H., Zhan, Z., Ma, K.-F., and Yeh, T.-Y. (2020). Modeling of
- pulse-like velocity ground motion during the 2018 Mw 6.3 Hualien earthquake, Taiwan.
- Geophys. J. Int., 223(1):348–365.
- Ling, S. and Okada, H. (1993). An extended use of the spatial autocorrelation method
- for the estimation of geological structure using microtremors. *Proceedings of the 89th*
- 562 SEGJ Conference, 44–48. (in Japanese).
- Lo, C.-L., Chang, E. T.-Y., and Chao, B. F. (2012). Relocating the historical 1951
- Hualien earthquake in eastern Taiwan based on tide gauge record. Geophys. J. Int.,
- 192(2):854-860.
- 566 Lo, Y.-C., Yue, H., Sun, J., Zhao, L., and Li, M. (2019). The 2018 Mw6.4 Hualien
- earthquake: Dynamic slip partitioning reveals the spatial transition from mountain
- building to subduction. Earth Planet. Sci. Lett., 524:115729.
- Martin, A. J. and Diehl, J. G. (2004). Practical experience using a simplified procedure
- to measure average shear-wave velocity to a depth of 30 meters (VS30). Proceedings of
- 13th World Conf. Earthq. Enq..
- Midorikawa, S. (1994). Site effects on strong-motion records observed during the 1987
- ⁵⁷³ Chiba-ken-toho-oki, Japan earthquake. Proc. Ninth Japan Earthq. Eng. Symposium,
- 1994, 3:85–90.
- Miyakoshi, K., Matsumoto, Y., Yamada, M., Mori, J., Cho, I., Hayashida, T., Kuo, C.-H.,
- Lin, C.-M., Yen, Y.-T., Kuo., K.-C. et al. (2019). Estimation of Underground Structures
- around Source Area of the 2018 Hualien Earthquake (Mw 6.4) using Microtremor Array

- Observations. Proceedings of the Fall meeting of the Seismological Society of Japan,
- Kyoto, Japan, 2019.9.
- Nakamura, Y. (1989). A method for dynamic characteristics estimation of subsurface using
- microtremor on the ground surface. Railway Technical Research Institute, Quarterly
- Reports, 30(1):25-33.
- Okamoto, T., Kokusho, T., Nishi, K., Tanaka, Y., Kudo, K., Suzuki, K., Kawai, T.,
- Sawada, Y., Ueshima, T., Kataoka, T., Yajima, H., Ikemi, M., and Higashi, S. (1998).
- Large-scale seismic test research at Hualien site in Taiwan results of site investigation
- and characterization of foundation ground. Central Research Institute of Electric Power
- Industry (CRIEPI) Research Report, U97062 (in Japanese).
- Parolai, S., Picozzi, M., Richwalski, S. M., and Milkereit, C. (2005). Joint inversion of
- phase velocity dispersion and H/V ratio curves from seismic noise recordings using a
- genetic algorithm, considering higher modes. Geophys. Res. Lett., 32(1):L01303.
- Picozzi, M., Parolai, S., and Richwalski, S. M. (2005). Joint inversion of H/V ratios
- and dispersion curves from seismic noise: Estimating the S-wave velocity of bedrock.
- ⁵⁹³ Geophys. Res. Lett., 32(11):L11308.
- Senna, S. and Fujiwara, H. (2008). Development of analyzing tools for microtremor survey
- observation data, vol. 1. Technical Note of the National Research Institute for Earth
- Science and Disaster Prevention, 313.
- Shin, T.-C., Chang, C.-H., Pu, H.-C., Hsiao-Wei, L., and Leu, P.-L. (2013). The geophys-
- ical database management system in Taiwan. Terr. Atmos. Ocean. Sci., 24(1):11.
- 599 Shyu, J. B. H., Chen, C.-F., and Wu, Y.-M. (2016). Seismotectonic characteristics of
- the northernmost Longitudinal Valley, eastern Taiwan: Structural development of a
- vanishing suture. Tectonophysics, 692:295–308.

- Tomioka, T. and Yamamoto, S. (2006). Development of low noise accelerometer (JA-
- 40GA). JAE Technical Report, (29):122–129 (in Japanese).
- Wang, X., Si, H., Koketsu, K., Nagano, M., and Dang, J. (2018). Building damage, strong
- ground motion characteristics and indoor damage of high-rise buildings in 2018 Hualien
- earthquake, Taiwan. Proceedings of the 15th Japan Earthquake Engineering Symposium,
- PS1-01-38 (in Japanese).
- Wen, Y.-Y., Wen, S., Lee, Y.-H., and Ching, K.-E. (2019). The kinematic source analysis
- for 2018 Mw 6.4 Hualien, Taiwan earthquake. Terr. Atmos. Ocean. Sci., 30:1–11.
- Wessel, P. and Smith, W. (1991). Free software helps map and display data. Eos,
- 72(441):445-446.
- ⁶¹² Wu, B.-L., Yen, J.-Y., Huang, S.-Y., Kuo, Y.-T., and Chang, W.-Y. (2019a). Surface
- deformation of 0206 Hualien earthquake revealed by the integrated network of RTK
- GPS. Terr. Atmos. Ocean. Sci., 30(3):301–310.
- ⁶¹⁵ Wu, Y.-M., Mittal, H., Huang, T.-C., Yang, B. M., Jan, J.-C., and Chen, S. K. (2019b).
- Performance of a low-cost earthquake early warning system (P-Alert) and shake map
- production during the 2018 Mw 6.4 Hualien, Taiwan, earthquake. Seismo. Res. Lett.,
- 90(1):19-29.
- Yu, S.-B., Chen, H.-Y., and Kuo, L.-C. (1997). Velocity field of GPS stations in the
- Taiwan area. Tectonophys., 274(1-3):41-59.

Table 1. Initial velocity structure for the inversion. The layer number, thickness, density, P-wave velocity, and S-wave velocity from the left.

| No | $\Delta H (m)$ | $\rho (\mathrm{g/cm^3})$ | Vp (m/s) | Vs (m/s) |
|----|----------------|---------------------------|----------|----------|
| 1 | 8 | 1.59 | 700 | 200 |
| 2 | 30 | 1.90 | 1400 | 300 |
| 3 | 30 | 2.02 | 1800 | 350 |
| 4 | 100 | 2.10 | 2100 | 600 |
| 5 | - | 2.17 | 2400 | 1000 |

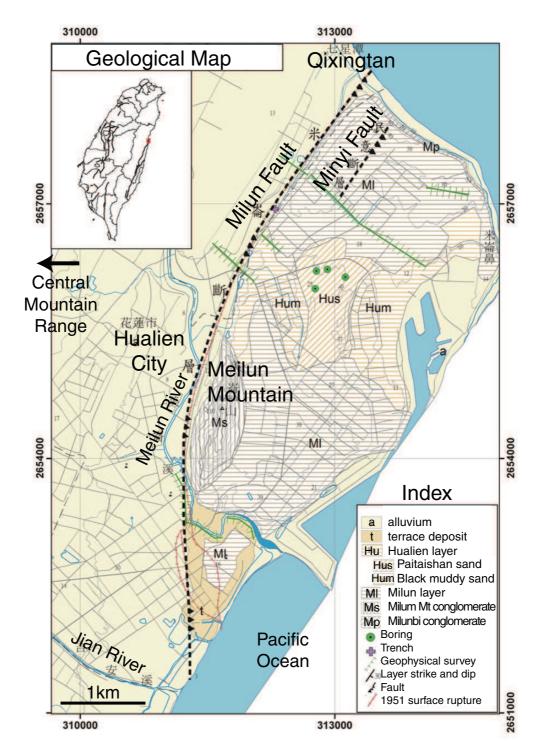


Figure 1. Geological map of the Hualien (modified after the Geological Map provided by Central Geological Survey, Taiwan. See Data and Resources Section). The coordinate system is TWD67 TM2.

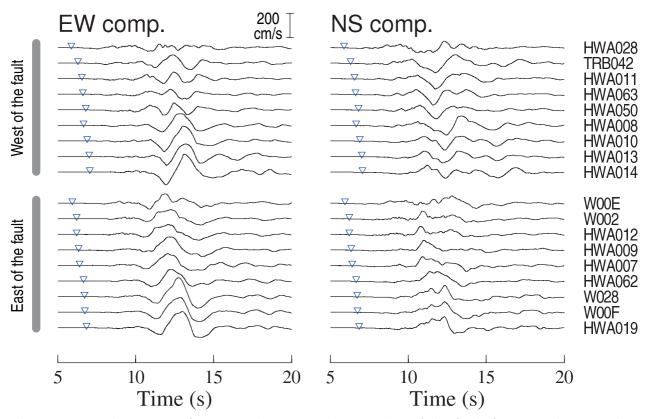


Figure 2. Velocity waveforms on the west and east sides of the fault from north to south. The inverted triangles show the theoretical S-wave arrival time. The horizontal axis shows the time after the origin time.

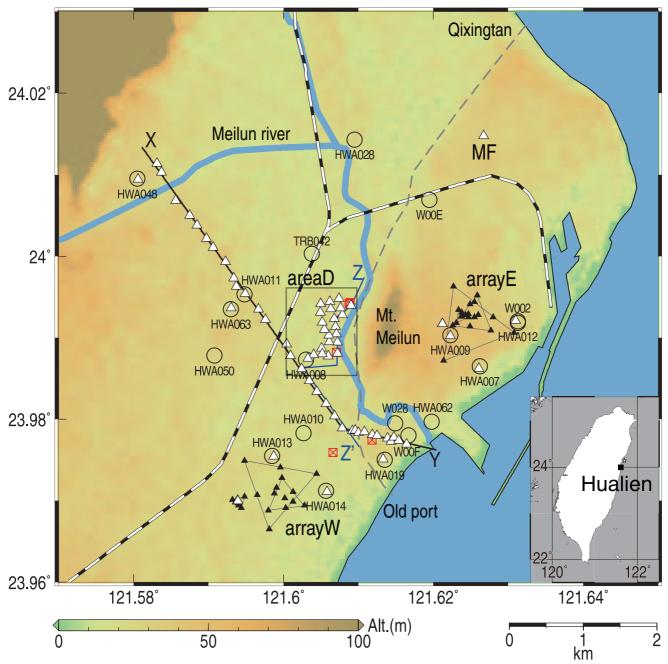


Figure 3. Map of the measurement locations. Open triangles show the locations of small arrays, and solid triangles show the locations of large arrays (array E and array W). Open circles show the locations of strong motion stations. Square symbols with a cross inside show the location of the heavily damaged buildings. Background color shows the altitude. The broken gray line shows the location of the Milun fault [Huang and Huang, 2018]. The railway is shown by a black and white line.

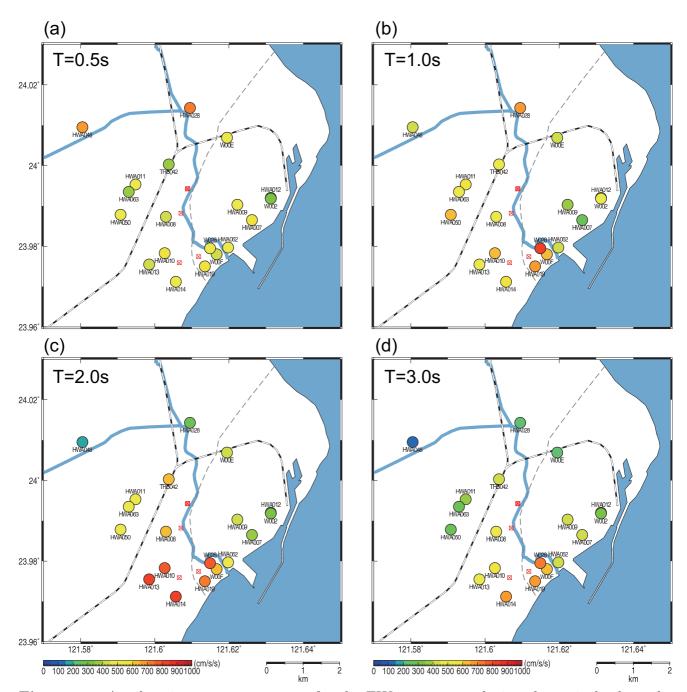


Figure 4. Acceleration response spectra for the EW component during the mainshock at the period of (a) 0.5 s, (b) 1.0 s, (c) 2.0 s, and (d) 3.0 s. The damping is 5 %. Other symbols are in the same format as Figure 3.

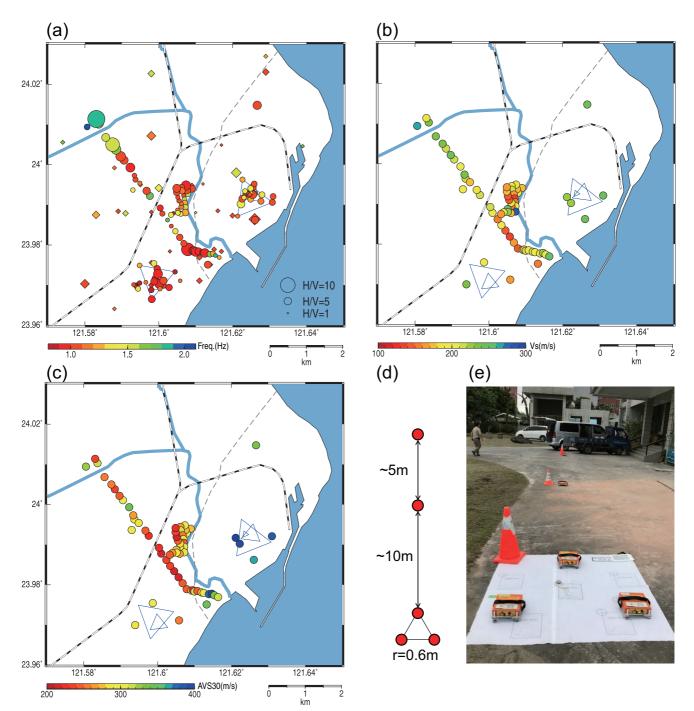


Figure 5. (a) Peak amplitudes and frequencies of the H/V spectrum. The size and color of the symbols show the peak amplitude and peak frequency, respectively. The circles show the results of this study, and the diamonds show the result of NCREE report (see Data and Resources Section). (b) S-wave velocity of the shallowest layer estimated from the phase dispersion curve. (c) AVS30 directly estimated from the phase velocity curves. (d) Sensor geometry for the small array measurement. (e) Photo of the small array measurement.

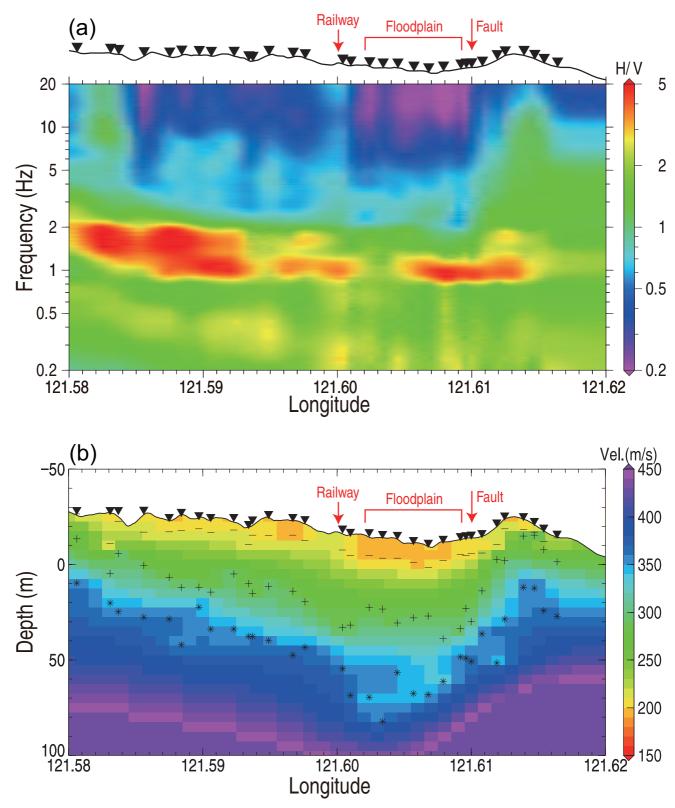


Figure 6. (a) H/V spectra along the X-Y section in Figure 3. The curves above the colored plots show the altitude and the triangles show the measurement location. (b) Inverted S-wave velocity structure along the X-Y section in Figure 3. Bars, crosses, and asterisks show the velocity structure boundary for the first, second and third layers, respectively.

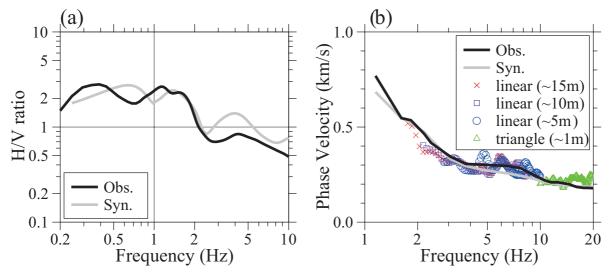


Figure 7. (a) Comparison between the observations (black) and synthetics (gray) for the H/V spectra. (b) Comparison of observed (black) and synthetic (gray) phase velocity curves at the station HWA011. The individual curves for arrays with different sizes are also shown with symbols. The frequency ranges corresponding to the wavelength of 3 – 20 times of the array radius are shown.

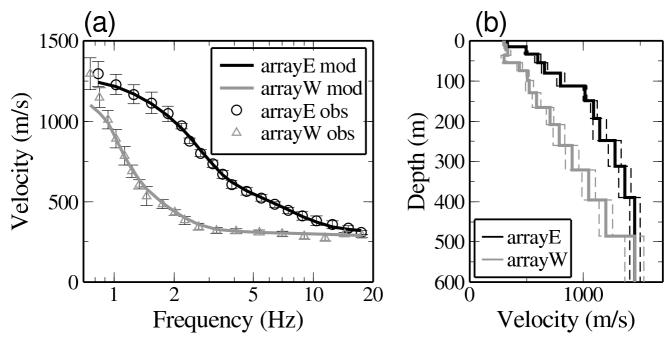


Figure 8. (a) Observed and estimated phase velocity curves for the large array E (black) and array W (gray). Errorbars for the observation are also shown. (b) Estimated velocity structure for the large array E (black) and array W (gray). Errors of the models are shown as thin dashed lines.

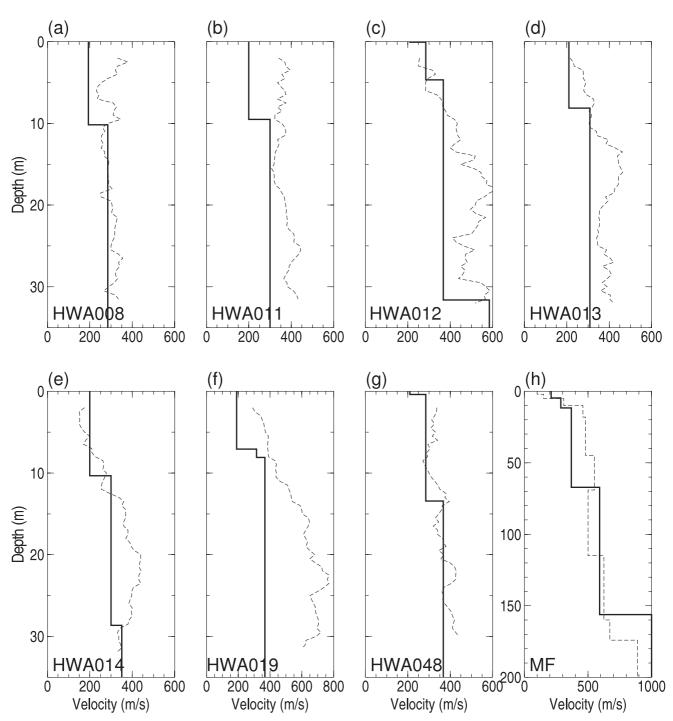


Figure 9. Velocity structures of the borehole logging data (dashed lines) and estimated velocity structures from the microtremor data (solid lines) at the strong motion stations: (a) HWA008, (b) HWA011, (c) HWA012, (d) HWA013, (e) HWA014, (f) HWA019, (g) HWA048, and (h) MF.

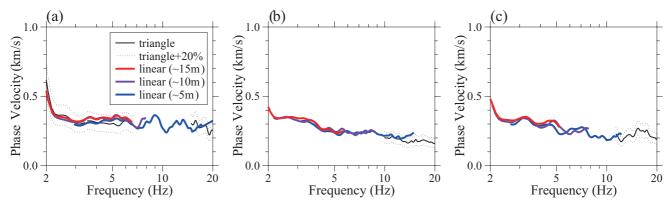


Figure 10. Observed phase velocity curves for the (a) large array W, (b) HWA008, and (c) HWA014. The thick black lines show the phase velocity curves estimated from the triangle array, and colored lines show those estimated from the linear array with two sensors. The broken lines show the range of $\pm 20\%$ from the estimation.

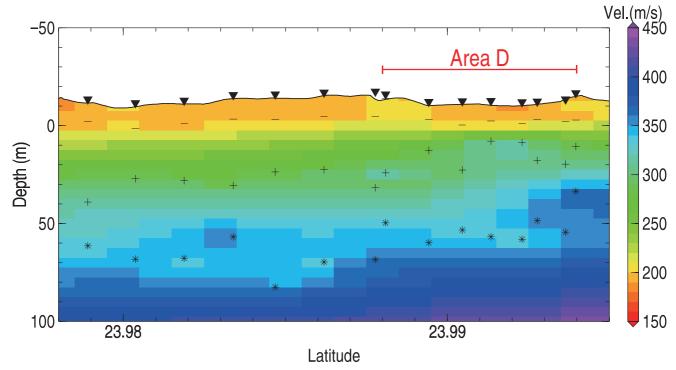


Figure 11. Inverted S-wave velocity structure along the Z-Z' section in Figure 3. The symbols are in the same format as Figure 6(b).

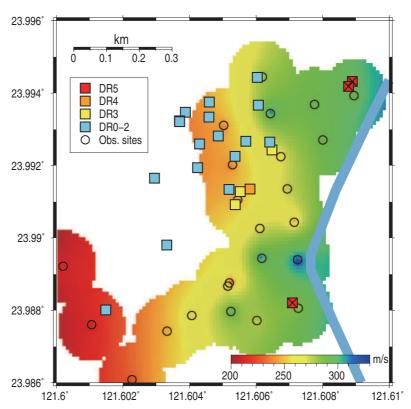


Figure 12. AVS30 (background color) and damage rank (square symbols) of the high-rise buildings in the heavily damaged area D. Open circles show the microtremor observation points. The thick line shows the Meilun river.